ADDITION @: 1776 LITCHFIELD RD SW SNELLVILLE, GA 30078

RELEASED FOR CONSTRUCTION

DRAWN BY: JASON ALBERT 678,390,4655 JASON@STUDIOTENDESIGNS.COM

GENERAL NOTES:

THE INFORMATION SHOWN IN THESE DRAWINGS IS BASED ON ACTUAL FIELD MEASUREMENTS AND OTHER INFORMATION OF RECORD. ALL WORK DESCRIBED IN THESE PLANS SHALL BE CONSTRUCTED IN COMPLIANCE WITH THE FOLLOWING CONSTRUCTION CODES.

THE GEORGIA STATE MINIMUM CODES

INTERNATIONAL BUILDING CODE - 2018 EDITION WITH GEORGIA STATE AMENDMENTS

INTERNATIONAL MECHANICAL CODE - 2018 EDITION WITH GEORGIA STATE AMENDMENTS INTERNATIONAL PLUMBING CODE - 2018 EDITION WITH GEORGIA STATE AMENDMENTS AND IPC APPENDIX F

INTERNATIONAL FUEL GAS CODE - 2018 EDITION WITH GEORGIA STATE AMENDMENTS

2020 NATIONAL ELECTRICAL CODE (NEC)

INTERNATIONAL ENERGY CONSERVATION CODE - 2015 EDITION WITH GEORGIA STATE AMENDMENTS

INTERNATIONAL RESIDENTIAL CODE FOR ONE & TWO FAMILY DWELLINGS, 2018 EDITION WITH GEORGIA STATE AMENDMENTS, AND IRC APPENDIX F

INTERNATIONAL FIRE PREVENTION CODE - 2018 EDITION WITH AMENDMENTS

THE GEORGIA EROSION AND SEDIMENTATION ACT OF 1975, THIRD EDITION 1992

NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) 2018 EDITION USED BY GA DCA

OCGA TITLE 25 AND 30 AND CHAPTER 120 OF THE FIRE COMMISONER'S RULES AND REGULATIONS

I'HIS PLAN HAS BEEN PREPARED TO MEET PROFESSIONAL STANDARDS AND PRACTICES. HOWEVER, BUILDING CODE REQUIREMENTS VERY WITH LOCATION AND CHANGE FROM TIME TO TIME. BEFORE STARTING CONSTRUCTION, THE CONTRACTOR MUST REVIEW AND BE RESPONSIBLE FOR ALL DIMENSIONS AND OTHER DETAILS AND SHOULD REVIEW PLANS TO INSURE THEY MEET CURRENT REQUIREMENTS. THE CONTRACTOR ASSUMES FULL RESPONSIBILITY TO VERIFY THE CONDITIONS, DIMENSIONS, AND STRUCTURAL DETAILS OF THE BUILDING, AND ASSUMES FULL LIABILITY FOR ANY PROBLEMS THAT MAY ARISE DUE TO POSSIBLE ERRORS ON THESE PLANS. ALL FEDERAL, STATE AND LOCAL CODES. ORDINANCES, REGULATIONS, ETC.,SHALL BE CONSIDERED AS PART OF SPECIFICATIONS FOR THIS BUILDING AND SHALL TAKE PRECEDENCE OVER ANYTHING SHOWN, DESCRIBED OR IMPLIED WHERE SAME ARE AT VARIANCE. USE OF THESE PLANS CONSTITUTES COMPLIANCE WITH THE ABOVE TERMS

THESE PLANS HAVE BEEN GENERATED FOR THE CLIENTS LISTED IN THE PROJECT NAME AND ARE NOT TO BE USED, REPRODUCED, COPIED, OR DISTRIBUTED FOR ANY OTHER PROJECT WITHOUT WRITTEN PERMISSION FROM STUDIO TEN DESIGNS.

- ALL MEANS AND METHODS OF CONSTRUCTION SHALL CONFORM TO CODES, LAWS, AND REGULATIONS OF GWINNETT COUNTY, INCLUDING BUT NOT LIMITED TO FLUES, CHIMNEY, FIREPLACE, SMOKE DETECTOR, MASONRY, WOOD CONSTRUCTION, ROOFING, PLUMBING, ELECTRICAL WIRING, EXHAUST FANS, VENTING, MECHANICAL EQUIPMENT, AND DUCTWORK, ETC., AND SUCH CODES, LAWS, AND REGULATIONS SHALL GOVERN OVER ANY CONFLICTING INFORMATION INDICATED ON THE CONSTRUCTION DOCUMENTS.
- THE DESIGNER SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS TECHNIQUES, SEQUENCES, OR PROCEDURES, OR SAFETY PRECAUTIONS IN CONNECTION WITH THE WORK, FOR ACTS OR OMISSIONS OF THE CONTRACTORS, SUBCONTRACTORS, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK OR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND / OR IN ACCORDANCE WITH LOCAL CODES, RESTRICTIONS, AND REQUIREMENTS.
- EACH NOTE ON ANY PAGE SHALL BE CONSIDERED AS ONE AND CONSISTENT FOR ALL PAGES.
- ALL PLAN DIMENSIONS ARE TO FACE OF FINISH PARTITIONS UNLESS OTHERWISE NOTED.
- ALL DIMENSIONS GOVERN OVER SCALE.
- CONTRACTOR TO CHECK AND VERIFY ALL CONDITIONS AND DIMENSIONS IN FIELD PRIOR TO CONSTRUCTION - NOTIFY DESIGNER OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION
- EACH BEDROOM SHALL HAVE AT LEAST ONE WINDOW WHOSE CLEAR OPENING IS A MINIMUM OF 5.7 SQ. FT. THE MINIMUM CLEAR WIDTH SHALL BE 20" AND MINIMUM CLEAR HEIGHT SHALL BE 24". GRADE FLOOR BEDROOM WINDOWS MAY HAVE A MINIMUM 5.0 SQ FT CLEAR OPENING

FOUNDATION WALLS:

- POURED CONCRETE FOUNDATION &/OR CMU WALLS SHALL BE MIN. NOMINAL 8" THICK AND STEEL REINFORCED AS NOTED ON DETAIL SECTIONS AND AS REQUIRED BY STATE, COUNTY, AND LOCAL CODES AND RESTRICTIONS.
- CONCRETE WALLS SHALL BE INSPECTED BY LICENSED ENGINEER OR ARCHITECT PRIOR TO POURING.
- WATERPROOFING ON CONC. WALLS MUST CONFORM TO LOCAL CODE REQUIREMENTS.
- USE 1/2" DIA. MIN. GALV. ANCHOR BOLTS OR STRAPS TO SECURE SILL PLATES 6'-0" O.C. AND A MAX 12" FROM CORNERS. PROVIDE FOAM SILL SEAL BETWEEN TOP OF FOUNDATION WALL AND SILL PLATE
- ALL PENETRATIONS THROUGH FOUNDATION WALLS MUST BE SEALED GAS TIGHT. B. PROVIDE FREE DRAINING GRANULAR BACKFILL WITH A MAX. EQUIV. FLUID PRESSURE OF 30 LBS PER
- SQ. FT. PER FOOT OF BACKFILL AGAINST FOUNDATION WALLS

ROOFING AND MOISTURE PROTECTION

- ALL METAL & SHINGLE ROOFING SYSTEM TO BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS AND ACCORDING TO THE GUIDELINES ESTABLISHED FOR CERTIFIED MFGR'S 20 YEAR NO DOLLAR LIMIT (NDL) WARRANTY.
- PROVIDE METAL DRIP CAP AT STARTER COURSES ABOVE GUTTERS
- PROVIDE FLASHING AT ALL DOORS, WINDOWS, AND OTHER OPENINGS AND AS NECESSARY AND AS PER CODE TO PREVENT MOISTURE PENETRATION.
- METAL FLASHING, COUNTER FLASHING, AND COPING SHALL BE MIN #26 GAUGE NON CORROSIVE METAL AND SHALL BE USED AT ALL STEPS, VALLEYS, AND COUNTERS
- MECHANICAL/PLUMBING/ ELECTRICAL CONTRACTORS SHALL BE REQUIRED TO SEAL ALL HORIZONTAL & VERTICAL PENETRATIONS IN THE EXTERIOR WALL CAUSED BY THEIR TRADE
- GENERAL CONTRACTOR IS RESPONSIBLE TO LOCATE AND PROVIDE NECESSARY STRUCTURAL. MECHANICAL

ELECTRICAL AND PLUMBING SLEEVES, ANCHORS, VENT OPENINGS ETC., THAT MIGHT BE REQUIRED.

- ALL WALL PLATES IN CONTACT W/ MASONRY OR CONC. SURFACE SHALL BE PRESSURE TREATED.
- ALL STUDS TO BE 2X4 OR 2X6 STUD GRADE SPF WITH 2" CDX PLYWOOD EXTERIOR SHEATHING OR EQUAL
- ALL JOISTS AND RAFTERS TO BE SPRUCE/PINE/FIR #2 AND BETTER. ROOF SHEATHING TO BE $\frac{1}{2}$ " THK. C.D.X. ALL FLOOR SHEATHING TO BE 3/4" T & G C.D.X. EXCEPT AREAS TO RECEIVE HARDWOOD FLOORING TO BE 1/2" C.D.X. PLYWOOD SUBFLOOR. ALL PLYWOOD SUBFLOOR TO BE GLUED TO JOISTS WITH APPROVED CONSTRUCTION ADHESIVE AND NAILED PER BLDG CODE.
- MANUFACTURED TRUSS JOIST SHALL BE INSTALLED IN ACCORDANCE WITH ALL MANUFACTURER'S SPECS TRUSS JOIST SHALL BE TRUSS JOIST MACMILLAN TJI-PRO 250 OR TJI PRO 350 OR EQUAL WITH RIM JOIST AS PER MFGR. SPECS. PROVIDE APPROVED CRUSH BLOCKS AT ALL POINT LOADS AND ALL BEARING POINTS AS RECOMMENDED BY MANUFACTURER

- PRECAST CONC, & LAMINATED WD BEAMS AND COLUMNS TO BE BUILT AND INSTALLED IN ACCORDANCE W/ ALL MANUFACTURER'S SPECIFICATIONS AND AS REQUIRED BY LOCAL CODES, RESTRICTIONS, AND REGULATIONS.
- PROVIDE APPROVED JOIST HANGERS AT ALL FLUSH JOIST-TO-JOIST AND JOIST-TO-BEAM CONNECTIONS HEADERS IN ALL BEARING PARTITIONS AND BEARING WALLS TO BE SOLID DIMENSIONAL LUMBER SIZED AS INDICATED ON FRAMING PLANS W/ SOLID PLYWOOD BETWEEN UNLESS OTHERWISE NOTED. LAMINATED
- 3. ALL HEADERS IN EXCESS OF 4'-0" SHALL HAVE MIN. (2) TRIMMER JACKS ON EACH SIDE

HEADERS AND BEAMS SHALL BE NAILED AS PER MANUFACTURER'S SPECIFICATIONS.

-). PROVIDE ADDITIONAL JOIST OR TRUSS UNDER INTERIOR PARTITIONS RUNNING PARALLEL TO FLOOR JOIST AND HAVING A LENGTH GREATER THAN 6'-0". DOUBLE JOIST UNDER BATHTUBS OR SPACE JOIST AT 12" O.C.
- 10. ALL BEARING PARTITIONS SHALL HAVE 2 TOP PLATES STAGGER SPLICES 4'-0" MIN. SPLICES SHALL BE CENTERED OVER TOP OF STUDS. STUDS SHALL ALIGN WITH JOISTS AND RAFTERS ABOVE AND BELOW
- PROVIDE 2X FIRESTOP BLOCKING AS REQUIRED BY CODE THROUGHOUT.
- . HOLES BORED OR CUT INTO JOISTS SHALL NOT OCCUR WITHIN 2" OF TOP OR BOTTOM OF JOISTS NOR IN CENTER ONE THIRD OF JOIST SPAN AND THE DIAMETER OF HOLES SHALL NOT EXCEED ONE THIRD OF THE DEPTH OF THE JOIST. NOTCHES SHALL NOT OCCUR IN TENSION SIDE OF JOIST. NOTCHES IN COMPRESSION SIDE OF JOISTS SHALL NOT OCCUR IN THE CENTER ONE THIRD OF THE SPAN AND SHALL NOT EXCEED ONE SIXTH OF THE
- . WHERE THE INSTALLATION OF PLUMBING, HEATING, OR OTHER PIPES NECESSITATES THE CUTTING OF TOP PLATES MORE THAN ONE HALF THEIR WIDTH A METAL TIE NOT LESS THAN 18 GAUGE AND 1 1/2" IN WIDTH SHALL BE FASTENED TO THE PLATE ACROSS AND TO EACH SIDE OF THE OPENING WITH NOT LESS THAN (4) 16 PENNY
- 14. THE DIAMETER OF HOLES BORED IN BEARING WALL STUDS SHALL NOT EXCEED ONE THIRD THE WIDTH OF THE STUD. WHERE STUDS ARE CUT OR BORED IN EXCESS OF ONE THIRD THE WIDTH OF THE STUD IT SHALL BE REINFORCED TO BE EQUAL IN LOAD CARRYING CAPACITY TO A STUD NOTCHED NOT MORE THAN ONE THIRD ITS DEPTH.
- STEEL LINTELS: (FOR EACH 4" THICKNESS OF MASONRY WALL)
- UP TO 3'-11" L3 \(\frac{1}{2}\)" X 3\(\frac{1}{2}\)" X 5/16 4'-0" TO 5'-11" L4" X 3¹/₂" X 5/16 6'-0" TO 7'-11" L5" X 3\frac{1}{2}" X 5/16 8'-0" TO 10'-0" W8X15 W/ SUSPENDED PLATE

2-2X6

- 3'-1" TO 5'-0" 2-2X8 5'-1" TO 6'-0" 2-2X10 6'-1" TO 7'-0" 2-2X12
- REINFORCED CMU LINTELS: PROVIDE A MINIMUM OF 8" BEARING AT EACH END LINTEL SIZE AND REINFORCING
- WALL THICKNESS X 8" DEEP, REINFORCED W/ 2#4 BOTTOM UP TO 8" THICK, REINFORCED W/3#4 BOTTOM OVER 8" THICK WALL THICKNESS X 16" DEEP, REINFORCED BOTTOM UP TO 8" THICK, REINFORCED W/ 3#5

PRECAST CONCRETE LINTELS: PROVIDE A MINIMUM OF 8" BEARING AT EACH END

BOTTOM OVER 8" THICK & #3 STIRRUPS @ 6" o.c.

- WALL THICKNESS X 8" DEEP, REINFORCED W/2#4 BOTTOM 4'-1" TO 8'-0" WALL THICKNESS X 16" DEEP, REINFORCED W/ 2#5 BOTTOM
- 16. $\,$ THE CONTRACTOR SHALL VERIFY ALL OPENINGS BELOW LINTELS INDICATED ARE ADEQUATE TO ACCEPT DOOR FRAMES, LOUVERS ETC. ARE SHOWN ON THE ARCHITECTURAL AND MECHANICAL DRAWINGS. NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES PRIOR TO LINTEL INSTALLATION.
- NO OPENINGS SHALL BE PLACED ABOVE ANY LINTEL WITHIN A HEIGHT LESS THAN OR EQUAL TO THE WIDTH OF THE CLEAR OPENING BELOW THE LINTEL, UNLESS SPECIFICALLY SHOWN OR APPROVED BY THE STRUCTURAL

FINISHES:

- 1. ALL EXTERIOR WOOD CORNICE AND TRIM SHALL BE PRIMED ON ALL SIDES PRIOR TO INSTALLATION
- 2. ALL INTERIOR WALLS AND CEILINGS TO BE $rac{1}{2}$ THICK GYPSUM WALLBOARD EXCEPT AS OTHERWISE NOTED.
- 3. SHOWER AND TUB WALLS ARE TO BE CERAMIC TILE ON CEMENTINOUS TILE BACKER BOARD.
- INTERIOR TRIM AND MOULDINGS INCLUDING BASE, CASINGS, CROWN, CHAIRRAIL, ETC. SHALL BE AS DETAILED AND/OR AS SELECTED BY OWNER

- INSULATION IN EXTERIOR WALLS, FLOORS, OR CEILINGS SHALL BE PAPER BACKED BLANKET OR ROLL TYPE FIBERGLASS WITH VAPOR BARRIER.
- INSULATION IN EXT. WOOD FRAME WALLS TO BE R-13 NOM. 3\(\frac{5}{8}\) AT 2X4 WALLS AND
- INSULATION IN FLAT CEILINGS ADJACENT TO ATTIC SPACES TO BE NOM. 10" (R-30)
- . PROVIDE R-13 INSULATION W/ FOIL VAPOR BARRIER AT CONC. FOUNDATION WALLS
- 5. NEW DOORS AND WINDOWS ARE REQ'D TO HAVE AN R-2.8 RATING MIN.

- UNLESS OTHERWISE NOTED, PROVIDE PERIMETER BASEMENT WALLS WITH 4" OR 6"G, DIAMETER PERFORATED, CORRUGATED PLASTIC DRAIN LAID ON 2" GRAVEL BASE W/ 6" -8" GRAVEL COVER WITH JOINTS COVERED WITH FILTER CLOTH FOR PERFORATED TILE. SLOPE DRAIN TILE AS REQUIRED TO DRAIN TO STORM SEWER OR OUTFALL.
- 3. PUT 18" OF GRAVEL ALL AROUND FOUNDATION.

DAMPPROOFING FOR CONCRETE AND MASONRY FOUNDATIONS

- I. EXTERIOR FOUNDATION WALLS OF CONSTRUCTION ENCLOSING BASEMENTS SHALL BE PORTLAND CEMENT PARGING TO THE WALL FROM FOOTING TO FINISH GRADE.
- THE PARING SHALL BE COVERED WITH A COAT OF APPROVED BITUMINOUS MATERIAL APPLIED AT THE RECOMMENDED RATE.

REINFORCING:

- REINFORCING STEEL SHALL BE HIGH STRENGTH NEW BILLET STEEL CONFORMING TO ASTM A615 -95C, GRADE 60 (60'000 PSI).
- WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A 185.
- : ALL REINFORCING SHALL BE DETAILED FABRICATED AND PLACED IN ACCORDANCE WITH THE ACI'S "MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES" (ACI - 315).
- . DETAILS OF REINFORCEMENT SHALL CONFIRM TO ACI 318 95, ACI 315 74 AND CRSI STANDARDS.
- ALL REINFORCING STEEL MARKED " CONTINUOUS " SHALL BE LAPPED 36 BAR DIAMETERS ST SPLICED AND AROUND CORNER OR INTERSECTION WITH A STANDARD 90 DEGREE BEND ON CORNER BARS.
- LAP WELDED WIRE MESH ONE FULL MESH AT SIDE AND END LAPS.
- SLABS ON GRADE SHALL BE 4" THK. CONCRETE AND REINFORCED WITH 6"X6" W1.4XW1.4 WWF LAP MESH 8" IN EACH DIRECTION. PLACE CONCRETE OVER 4 MIL. POLYETHYLENE VAPOR BARRIER AND 4" MINIMUM OF COARSE AGGREGATE OR AS RECOMMENDED BY SOILS ENGINEER. THE AGGREGATE LAYER SHALL BE PLACED OVER FIRM NATURAL SUB GRADE OR ON COMPACTED OR AND CONTROLLED FILL. FILL UNDER SLABS SHALL BE COMPACTED IN 8" LAYERS TO 95% MAXIMUM DENSITY. USE AIR ENTRAINED CONCRETE AT ALL EXTERIOR SLABS. POUR SLABS IN ALTERNATE PANELS WITH MAXIMUM OF 600 SQUARE FEET AND PROVIDE CONTROL & CONSTRUCTION JOINTS AT 30'-0" MAXIMUM OR AS REQUIRED TO PREVENT UNCONTROLLED CRACKING.

Birdsong Agency OCCUPANCY TYPE: EXISTING CONSTRUCTION (RESIDENTIAL) NO. OF STORIES: 2 OVER SLAB Fantasy MedSpa / Lash Fantasy Webb Gin House Rd THIS BUILDING WAS DESIGNED IN ACCORDANCE WITH THE INTERNATIONAL RESIDENTIAL CODE FOR SINGLE-FAMILY DWELLINGS - 2018 EDITION WITH Iglesia Cristian ynamic Styles 2020 GEORGIA STATE AMENDMENTS Monte de Sio Hair Studio Paola Cuartas - State REAR ADDITION TO MAIN FLOOR OF HOME Farm Insurance Agen RFAR ADDITION TO BASEMENT LEVEL OF HOME REAR ADDITION TO UPPER FLOOR OF HOME REMODEL EXISTING INTERIOR ville, GA 30078 FRONT PORCH ADDITION REAR PORCH ADDITION

LOCATION MAP

SYMBOLS:

100 BEDROM

ROOM NUMBER & TITLE

MISCELLANEOUS

MASONRY OPENING

METAL THRESHOLD

NOT IN CONTRACT

SHEET INDEX:

CS COVER SHEET ARCHITECTURAL:

- A1 EXISTING FOUNDATION PLAN
- A2 EXISTING FLOOR PLANS A3 EXISTING ROOF & ROOF FRAMING PLAN
- A4 EXISTING CEILING FRAMING PLANS A5 EXISTING EXTERIOR ELEVATIONS
- A6 PROPOSED FOUNDATION PLAN
- A7 PROPOSED FLOOR PLANS A8 PROPOSED FRONT & REAR ELEVATIONS
- A9 PROPOSED RIGHT & LEFT SIDE ELEVATIONS
- A10 PROPOSED ROOF & ROOF FRAMING PLAN A11 PROPOSED CEILING FRAMING PLANS
- A12 PROPOSED DECK FRAMING PLANS
- A13 PROPOSED SECTIONS & DETAILS A14 PROPOSED SECTIONS & DETAILS
- A15 PROPOSED FRAMING CALCS
- A16 PROPOSED FRAMING CALCS
- A17 PROPOSED FRAMING CALCS
- SQ. FT. DATA **EXISTING (HEATED)**
- PROPOSED (HEATED) **NEW FRONT PORCH NEW REAR PORCH**

2 SOH

◆ 9'-6" AFF

W/O

ROUND

ROUGH OPENING

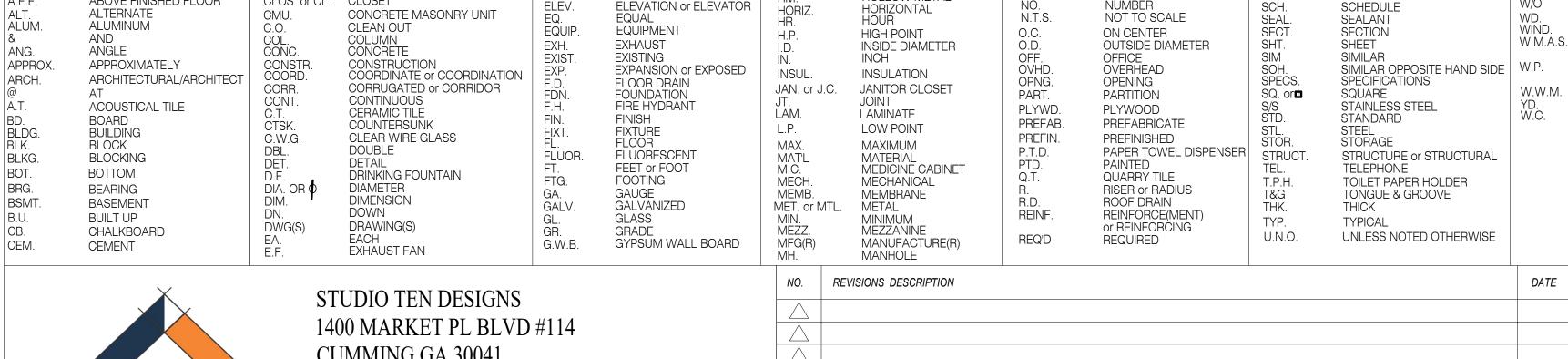
1,486 SQFT 2,519 SQFT 125 SQFT 183 SQFT

KEYED NOTE NORTH ARROW PARTITION TYPE CONCRETE BLOCK PLYWOOD or PARTICLE **EXISTING CONSTRUCTION** WALL SECTION **NEW CONSTRUCTION ELEVATION** OR FILLED BLOCK DEMOLITION DETAIL EXPANSION JOINT MATERIAL COLUMN CENTERLINE GRAVEL or CRUSHED STONE BATT INSULATION **ENLARGED DETAIL ROOF PLANK** WINDOW SYMBOL **ELEVATION** GYPSUM BOARD or DOOR SYMBOL

HOSE BIBB

HOLLOW METAL

HEAD



GYPSUM DECK

EXTERIOR INSULATION

FOR FXAMPI F

ELECTRIC(AL)

EXPANSION JOINT

E.I.F.S.



COMPOSITION TILE

AIR CONDITIONING

ABOVE FINISHED FLOOR

ROUGH WOOD CONTINUOUS

CLOS. or CL.

MATERIAL SCHEDULE:

CUMMING GA 30041

Studio Ten PHONE: 678.390.4655 EMAIL: JASON@STUDIOTENDESIGNS.COM

CONTROL JOINT

CENTERLINE

CLOSET

PERMIT ISSUE:

CEILING HEIGHT

WITHOUT

SHELVES

PROJECT#

WEATHERPROOF or WATERPROOF

WATER CLOSET or

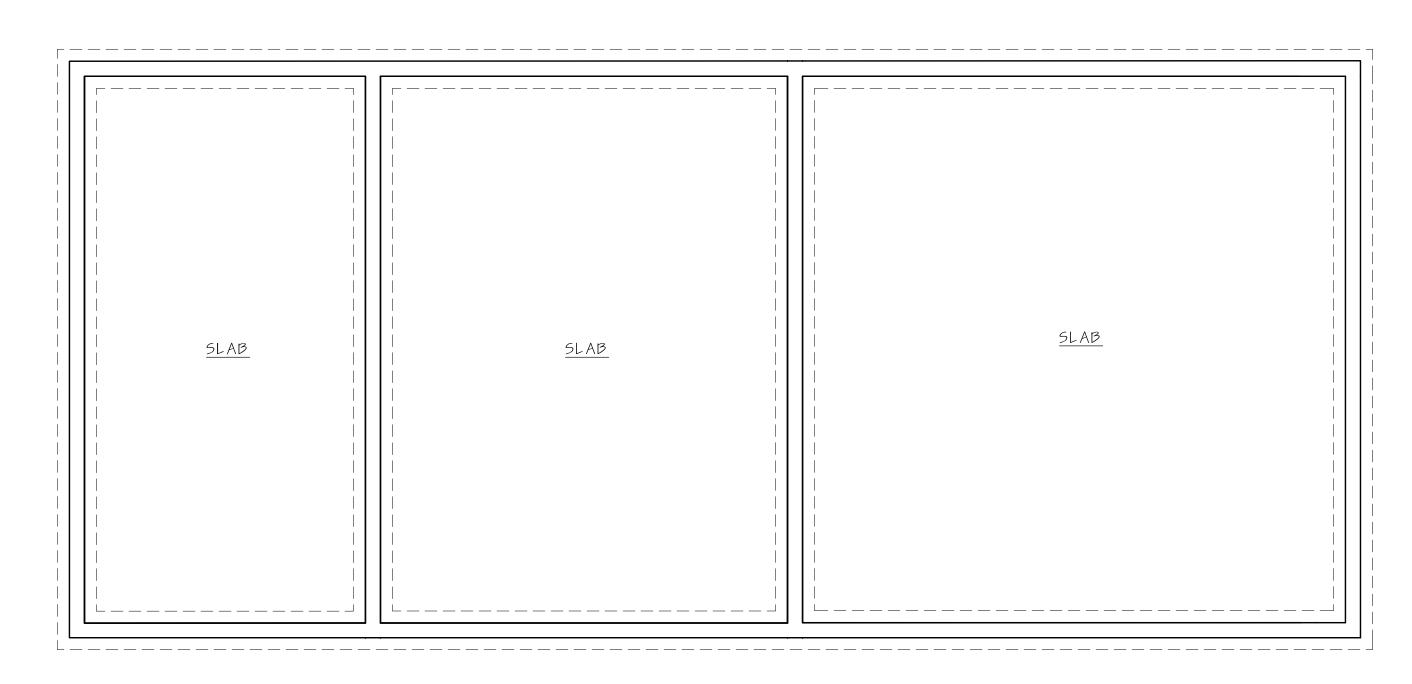
WALL COVERING

WELDED WIRE MESH

VINYL COMPOSITION TILE VERIFY IN FIELD

WALL MOUNTED ADJUSTABLE

11.24.21



EXISTING FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

ENGINEER STAMP & SEAL



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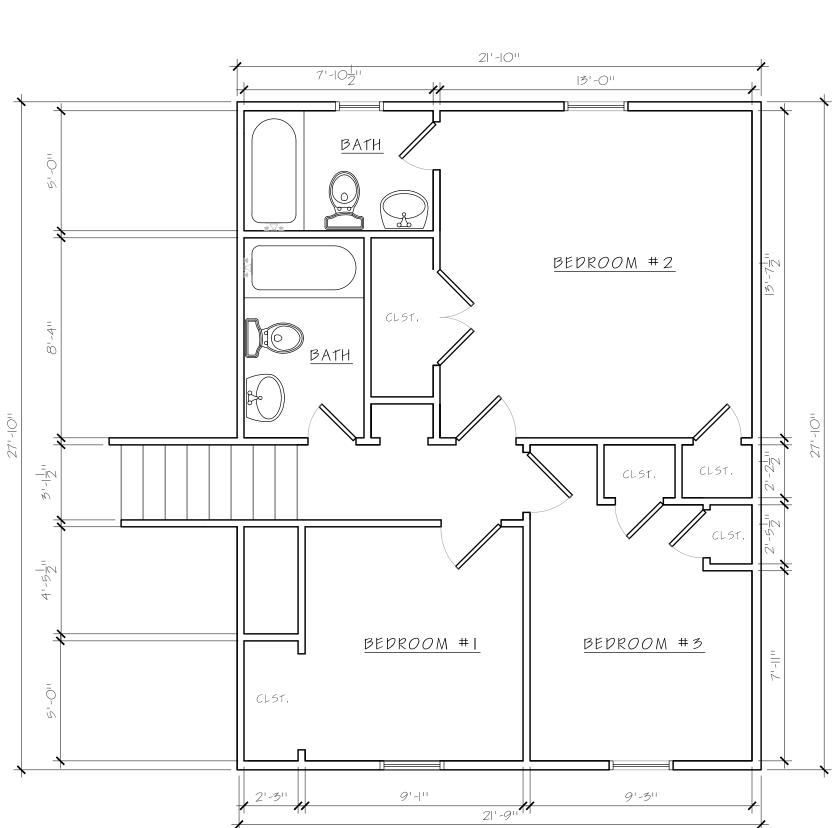
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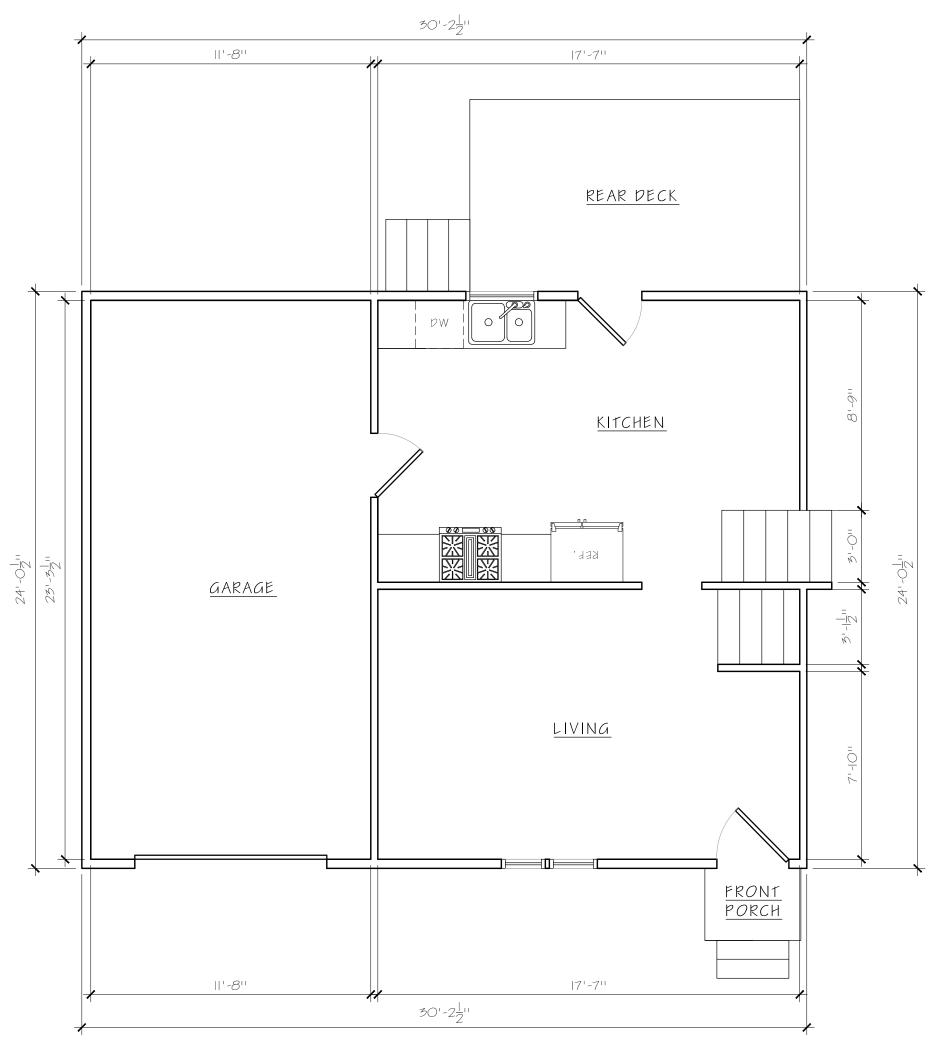
SEPTEMBER 23, 2021

SHEET TITLE: EXISTING

FOUNDATION PLAN

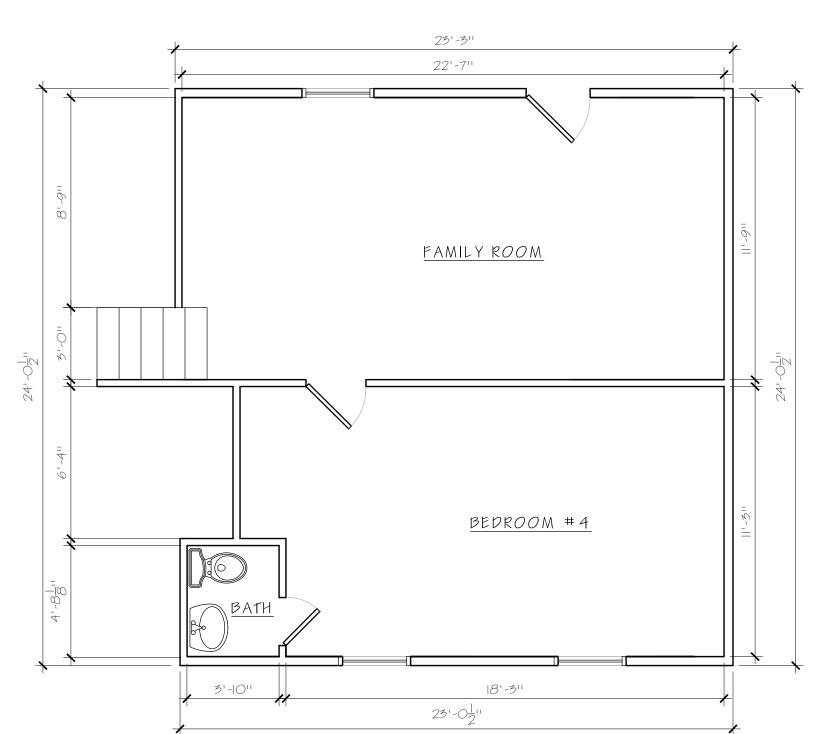






EXISTING MAIN FLOOR PLAN

SCALE: 1/4" = 1' - 0"



EXISTING BASEMENT FLOOR PLAN SCALE: 1/4" = 1' - 0"

ENGINEER STAMP & SEAL



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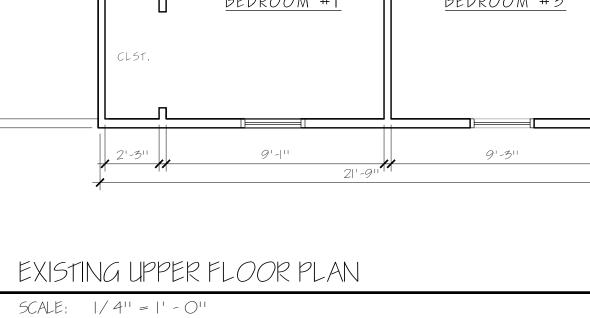
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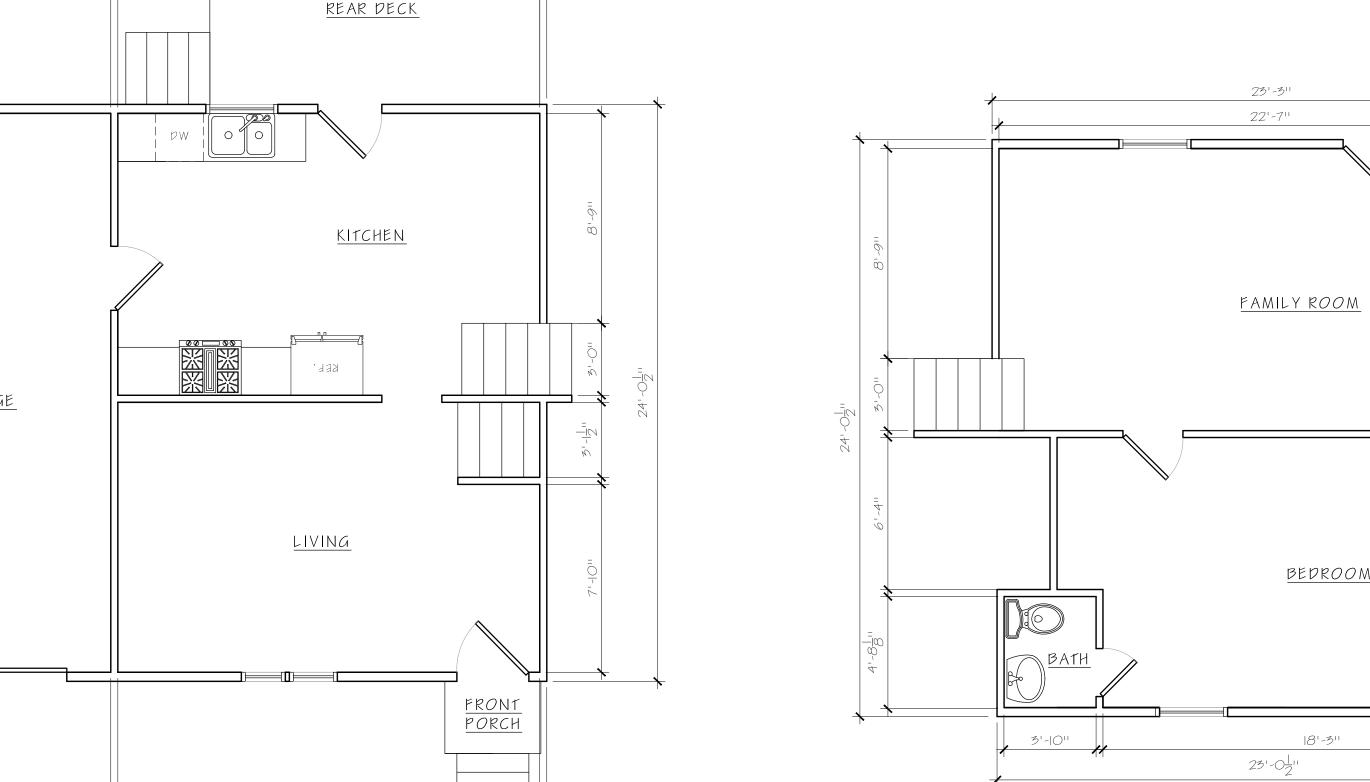
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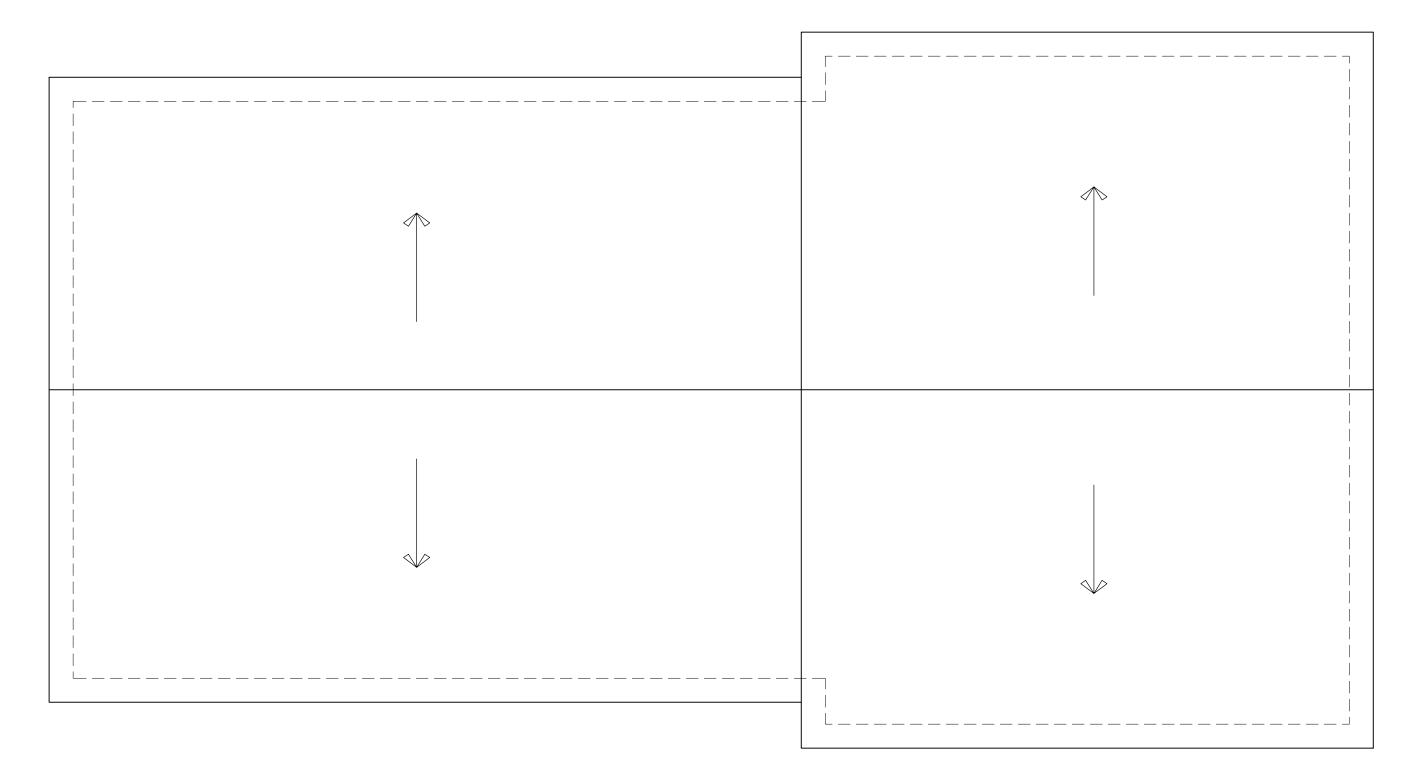
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EXISTING FLOOR PLANS

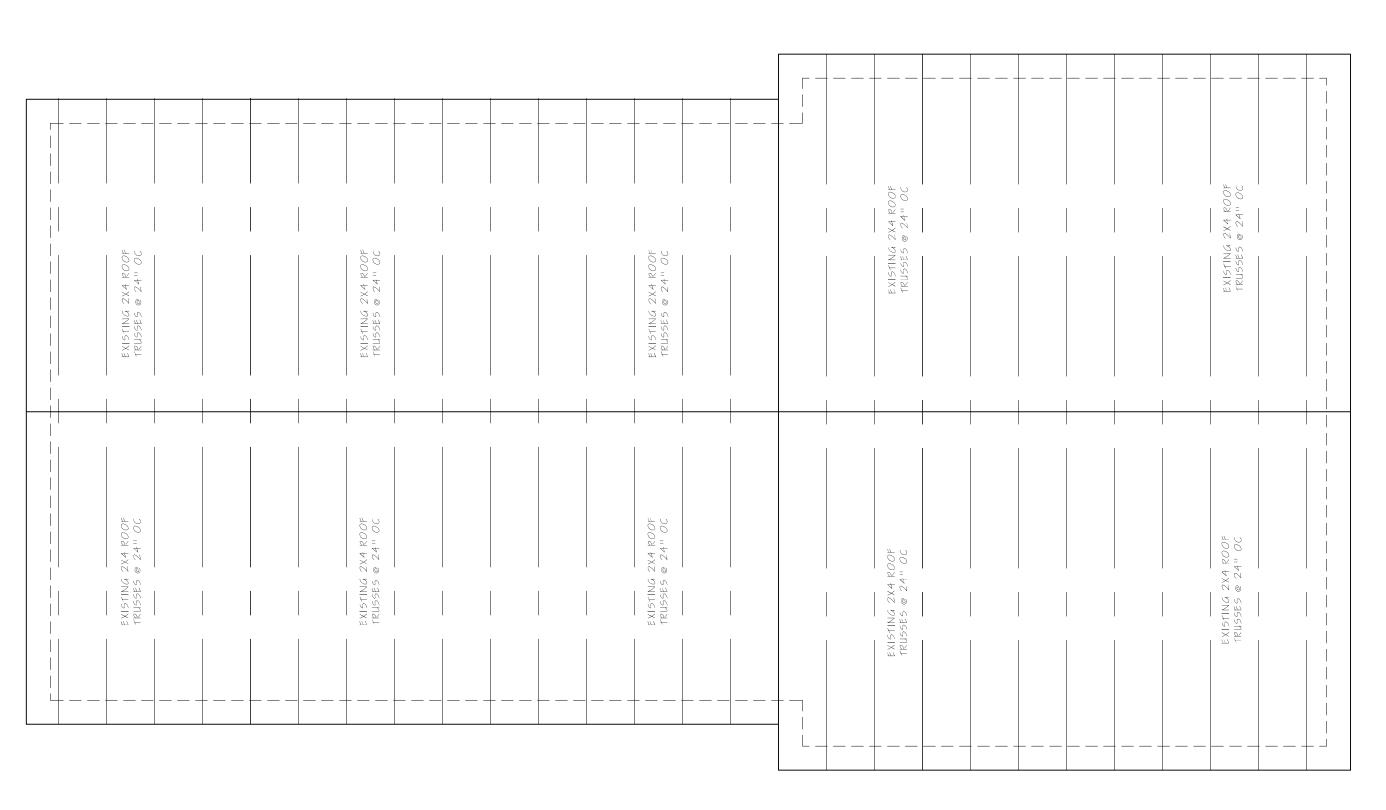






EXISTING ROOF PLAN

SCALE: 1/4" = 1'-0"



EXISTING ROOF FRAMING PLAN

SCALE: 1/4" = 1' - 0"

ENGINEER STAMP & SEAL



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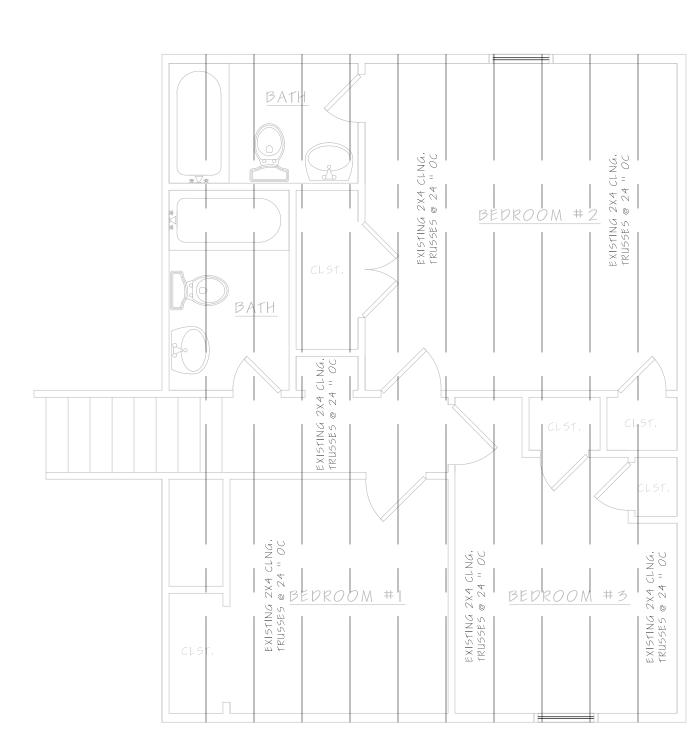
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SEPTEMBER 23, 2021 SHEET TITLE:

EXISTING ROOF & ROOF FRAMING PLAN



EXISTING UPPER CLNG, FRMG, PLAN

REAR DECK FRONT PORCH EXISTING MAIN CLNG, FRMG, PLAN

EXISTING BASEMENT CLNG. FRMG. PLAN 9 SCALE: 1/4" = 1' - 0"

ENGINEER STAMP & SEAL



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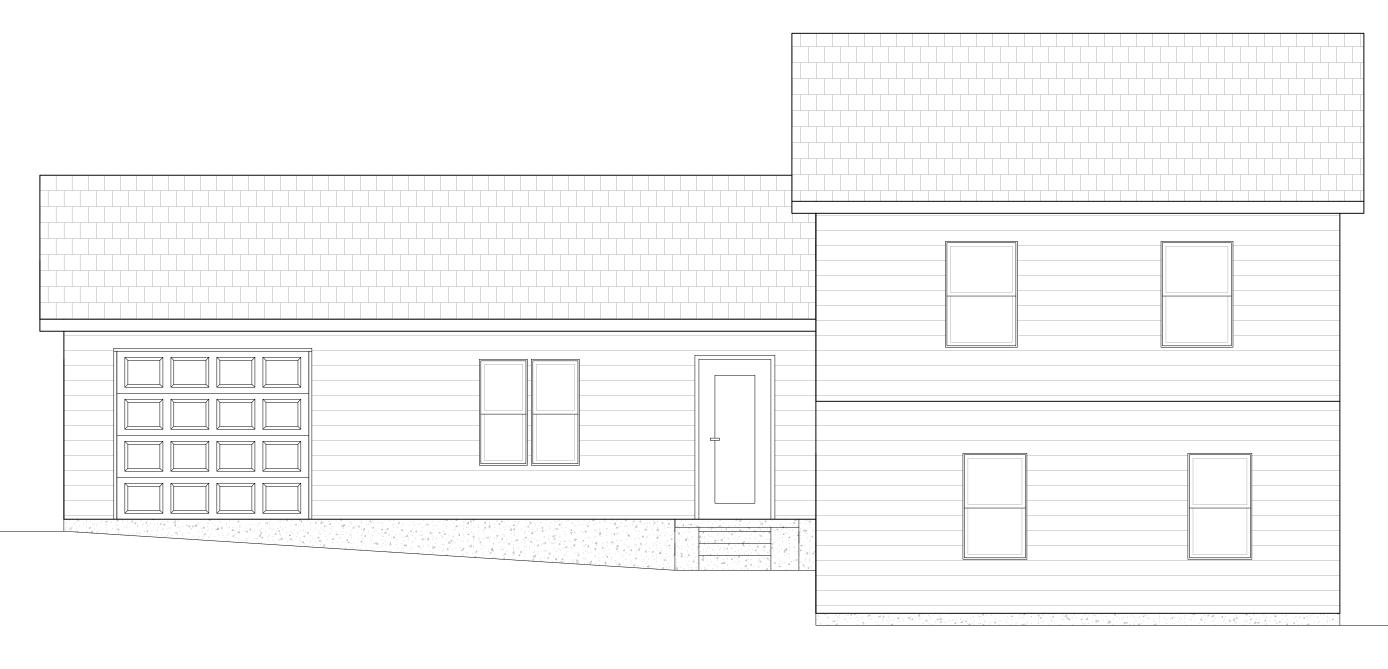
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SEPTEMBER 23, 2021 SHEET TITLE:

EXISTING CLNG, FRMG. PLANS



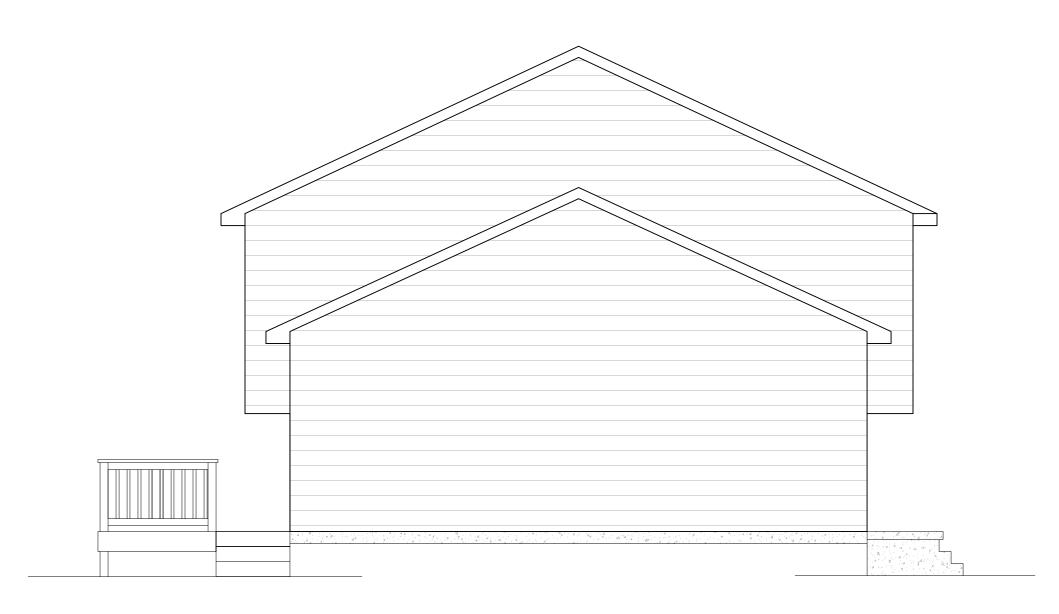


EXISTING FRONT ELEVATION



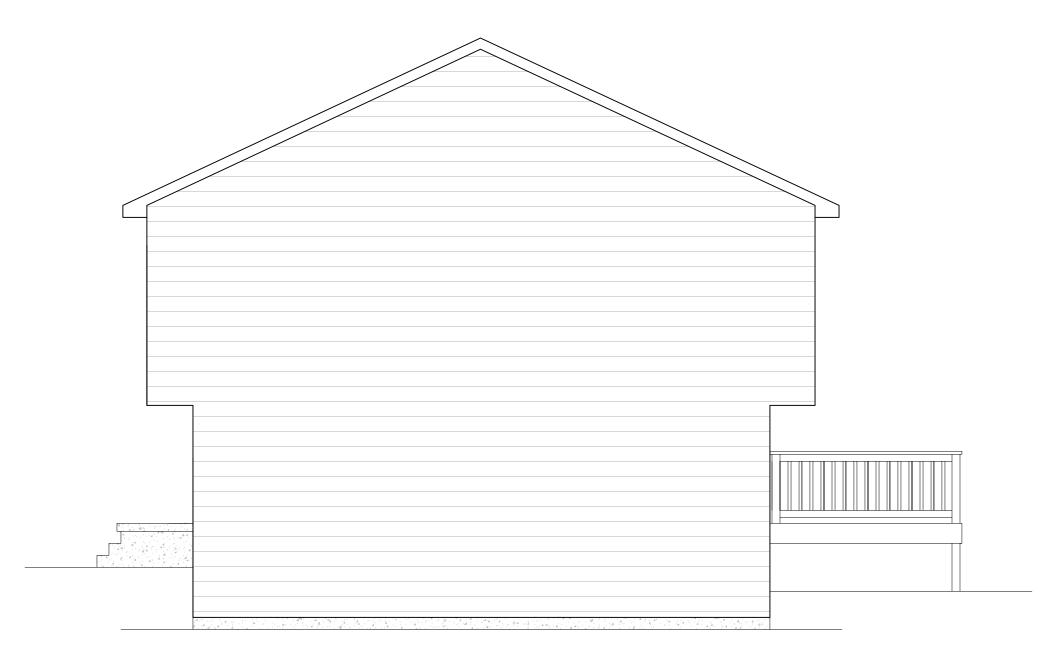
EXISTING REAR ELEVATION

SCALE: 1/4" = 1'-0"



EXISTING LEFT SIDE ELEVATION

SCALE: 1/4" = 1' - 0"



EXISTING RIGHT SIDE ELEVATION

ENGINEER STAMP & SEAL



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SEPTEMBER 23, 2021 SHEET TITLE:

> EXISTING EXTERIOR ELEVATIONS

PROPOSED FOUNDATION PLAN

ENGINEER STAMP & SEAL



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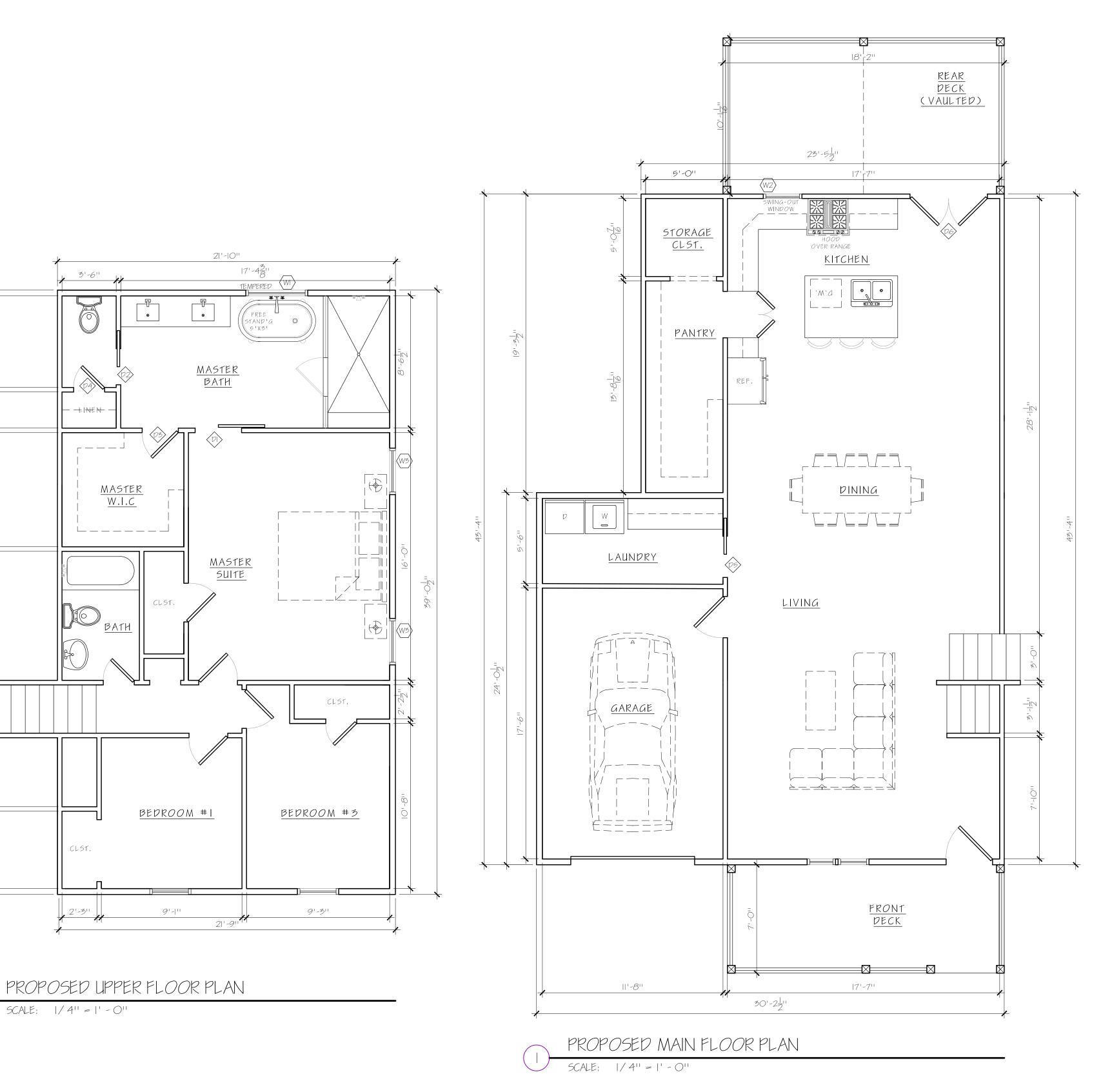
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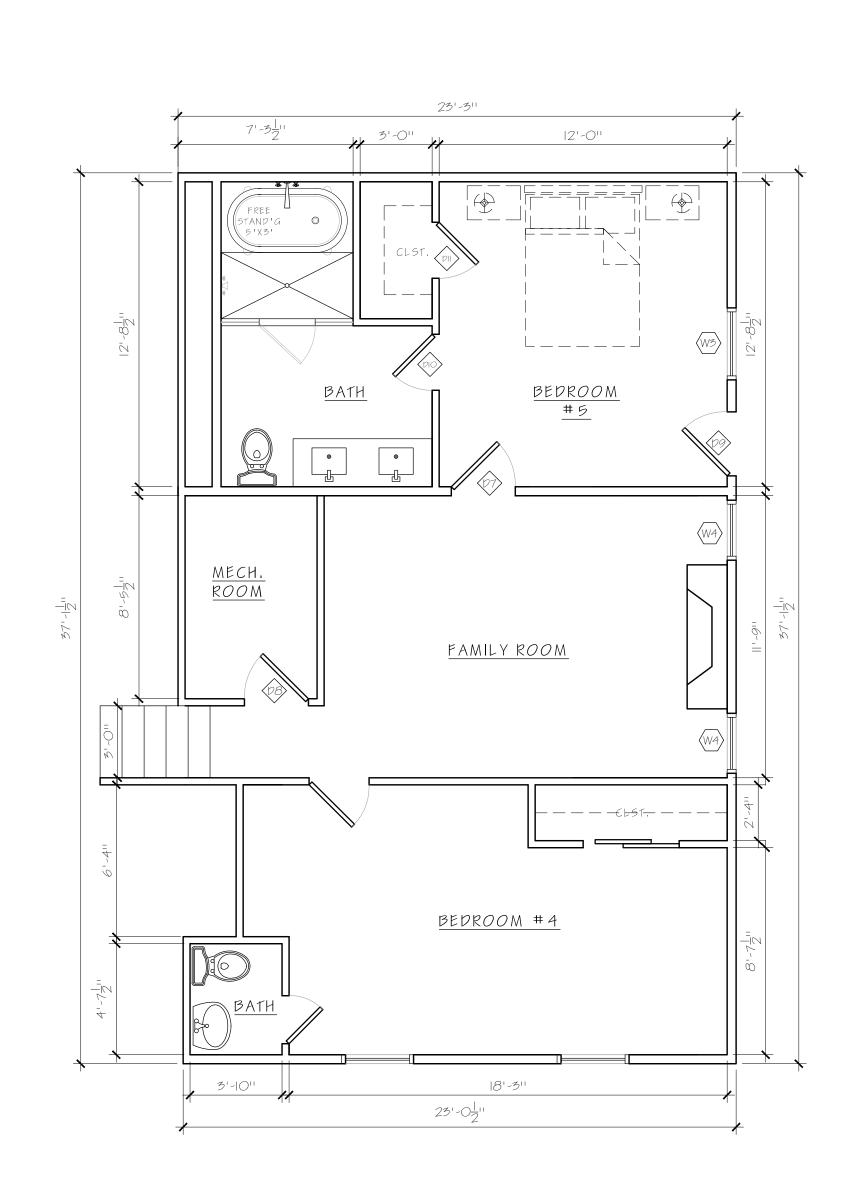
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SEPTEMBER 23, 2021 SHEET TITLE:

PROPOSED

FOUNDATION PLAN





PROPOSED BASEMENT FLOOR PLAN

SCALE: 1/4" = 1' - 0"

ENGINEER STAMP & SEAL



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SEPTEMBER 23, 2021 SHEET TITLE:

> PROPOSED FLOOR PLANS

A 7

PROPOSED FRONT ELEVATION SCALE: 1/4" = 1' - 0"



PROPOSED REAR ELEVATION

SCALE: 1/4" = 1'-0"

ENGINEER STAMP & SEAL



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REMODEL @
1776 LITCHFIELD RP SV
SNELLVILLE, GA 2007

PROJECT MANAGER

<u>DRAWN BY:</u> JASON ALBERT 678,390,4655 JASON@STUDIOTENDESIGNS.COM

DOCUMENT PHASE:

RELEASED FOR CONSTRUCTION

SEPTEMBER 23, 2021 SHEET TITLE:

PROPOSED FRONT & REAR ELEVATIONS

ENGINEER STAMP & SEAL:



STUDIO TEN DESIGNS



JASON ALBERT - 678.390.4655 JASON@STUDIOTENDESIGNS

REMOPEL 1776 LITCHFIELD R SNELLVILLE, GA 30

PROJECT MANAGER

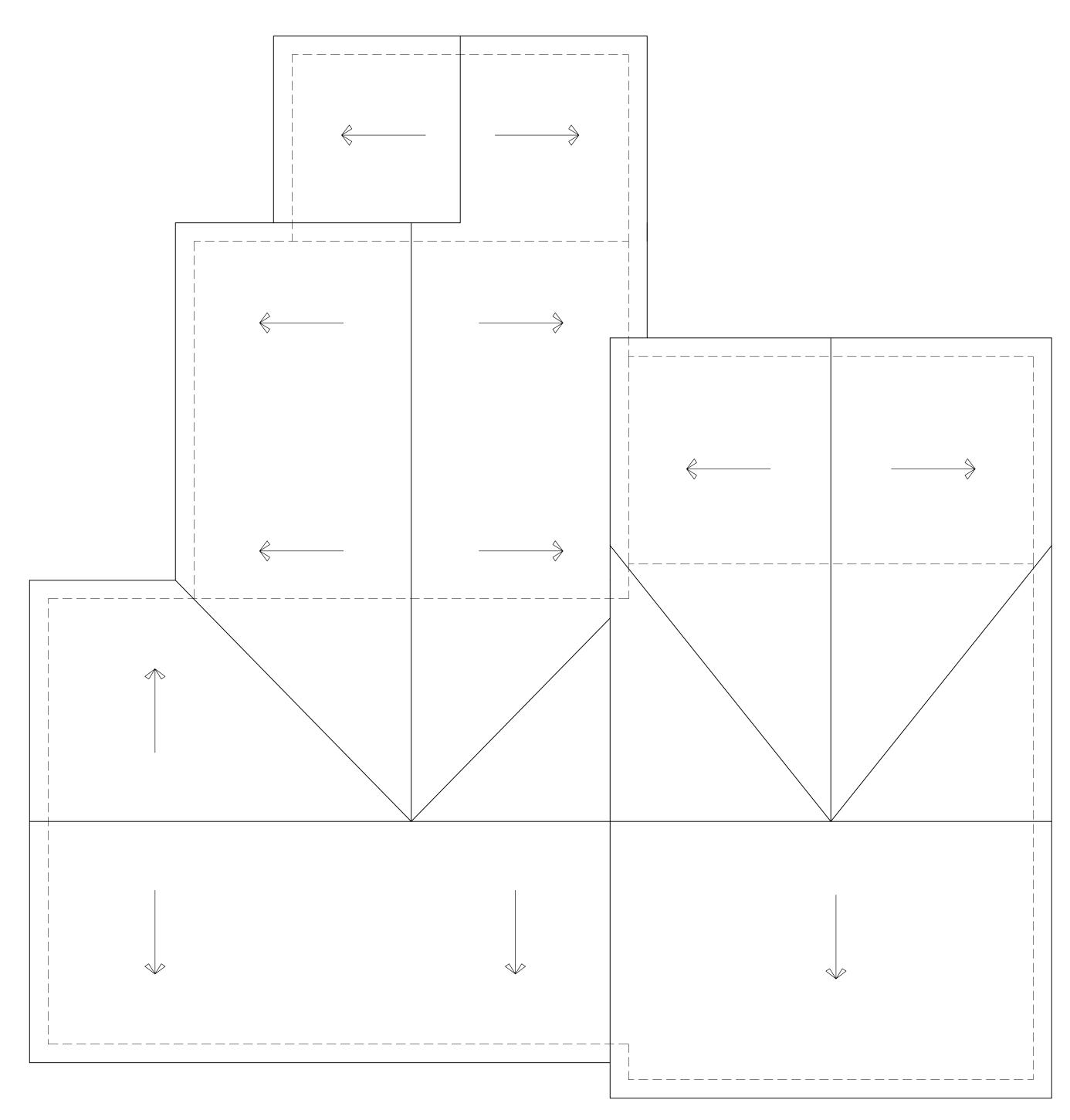
<u>DRAWN BY:</u> JASON ALBERT 678.390.4655 JASON@STUDIOTENDESIGNS.COM

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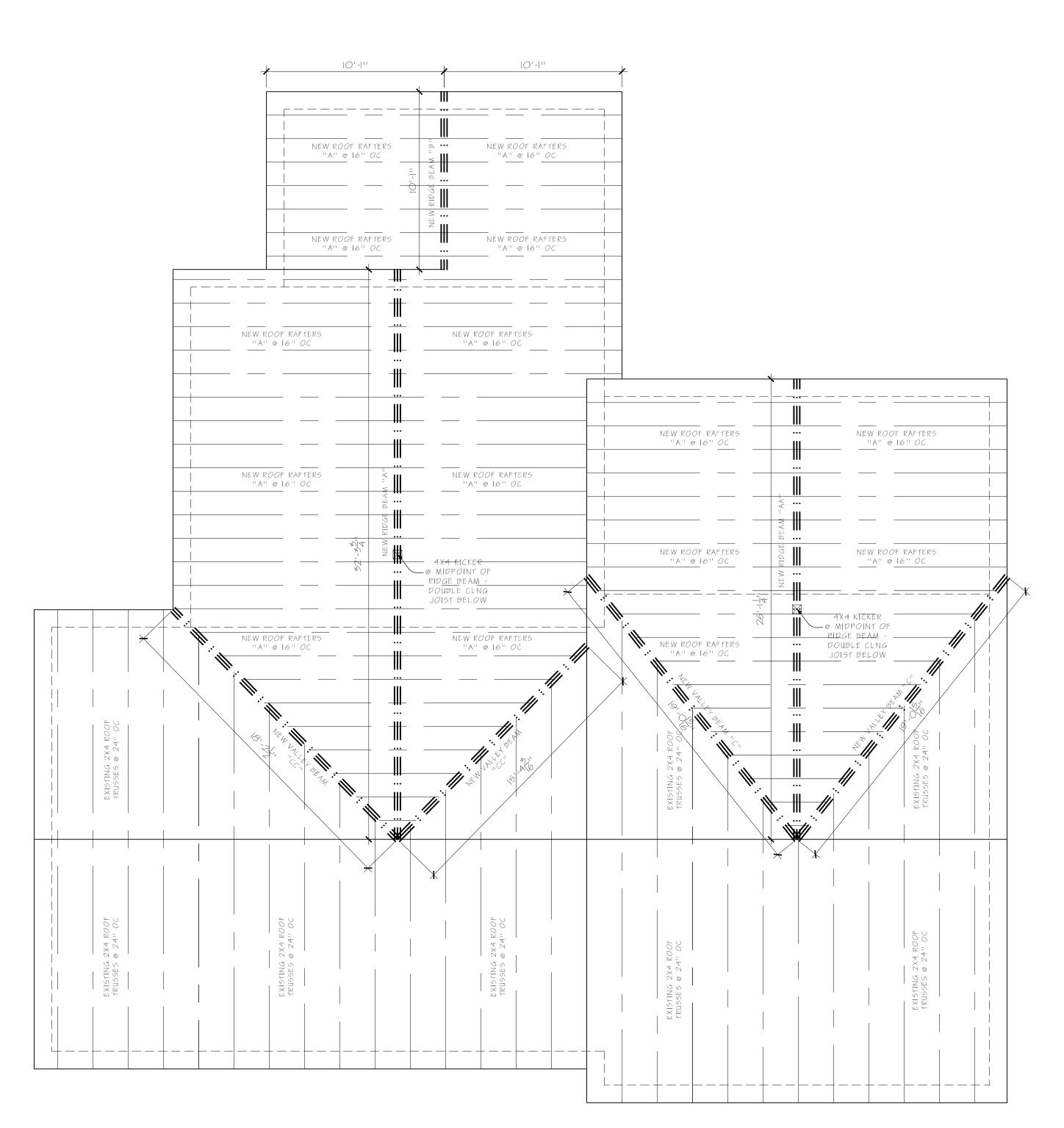
SEPTEMBER 23, 2021 SHEET TITLE:

PROPOSED RIGHT & LEFT SIDE ELEVATIONS



PROPOSED ROOF PLAN

SCALE: 1/4" = 1' - 0"



PROPOSED ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

ENGINEER STAMP & SEAL



STUDIO TEN DESIGNS



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REMODEL @
1776 LITCHFIELD RD SW
SNELLVILLE, GA 20078

PROJECT MANAGER

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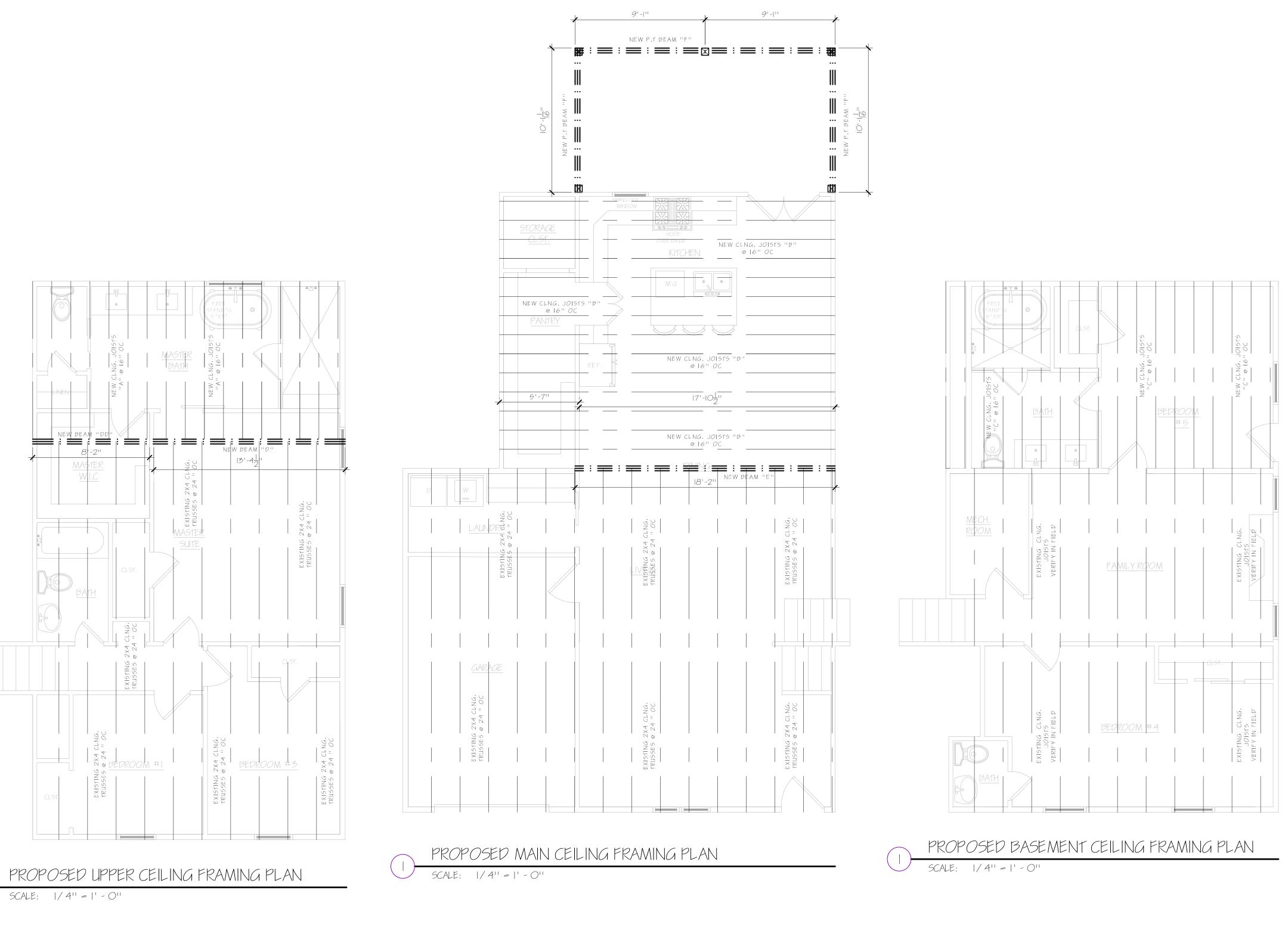
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SEPTEMBER 23, 2021 SHEET TITLE:

PROPOSED ROOF & ROOF FRAMING PLANS

A



ENGINEER STAMP & SEAL:



STUDIO TEN DESIGNS



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REMODEL @
1776 LITCHFIELD RD SW
SNELLVILLE, GA 30078

PROJECT MANAGER

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DOCUMENT PHASE:

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SEPTEMBER 23, 2021 SHEET TITLE:

PROPOSED CEILING FRAMING PLANS

A.

PROPOSED DECK FRAMING PLANS

SCALE: 1/4" = 1' - 0"

GENERAL REQUIREMENTS FOR DECK:

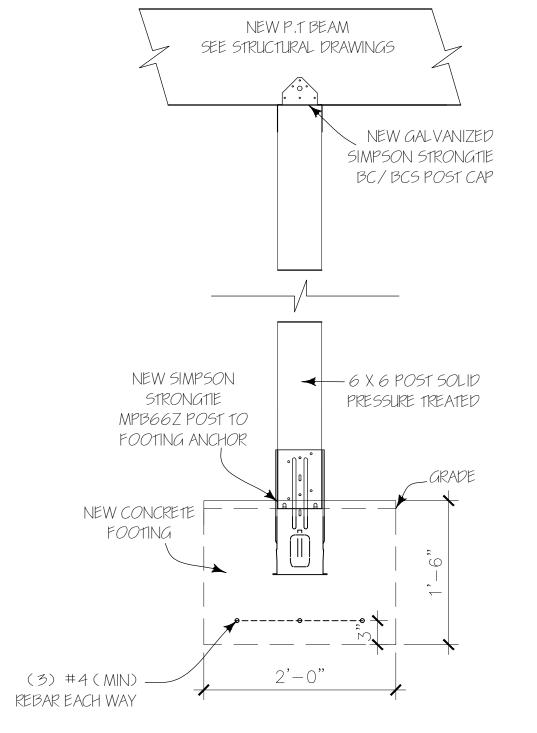
I. LUMBER SHALL BE NATURALLY DURABLE WOOD OR SHALL BE SOUTHERN PINE, GRADE #2 OR BETTER THAT IS PRESSUREPRESERVATIVE-TREATED

IN ACCORDANCE WITH AWPA UL FOR THE SPECIES, PRODUCT, PRESERVATIVE AND END USE.

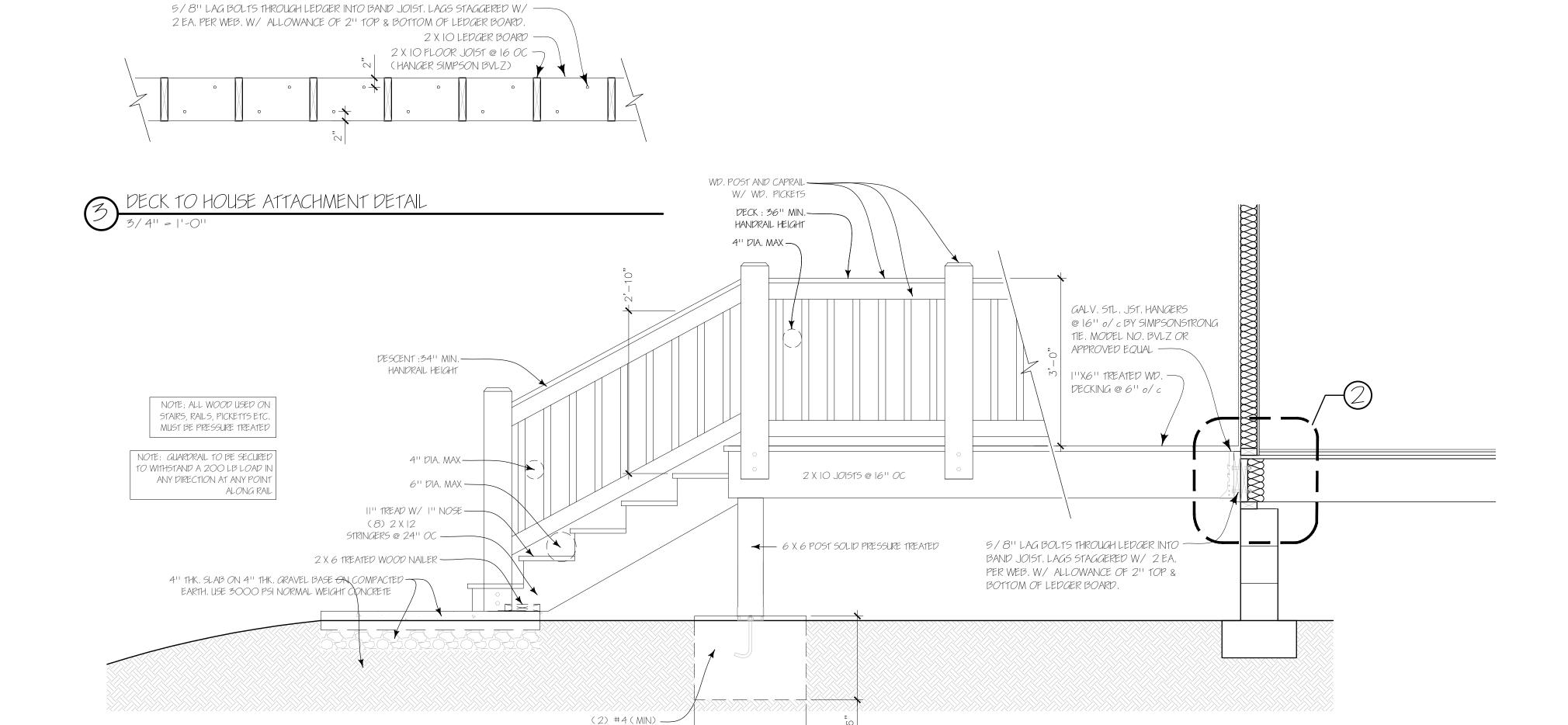
FIELD CUT ENDS, NOTCHES AND DRILLED HOLES OF PRESERVATIVE TREATED WOOD SHALL BE TREATED IN THE FIELD IN ACCORDANCE WITH AWPA M4. PRESERVATIVE - TREATED LUMBER IN CONTACT WITH THE GROUND SHALL BE RATED AS "GROUND-CONTACT." PLEASE NOTE: NOT ALL TREATED LUMBER IS RATED FOR GROUND CONTACT.

2. WOOD-PLASTIC COMPOSITES ARE COMPOSED OF BOUND WOOD AND PLASTIC FIBERS CREATING MATERIAL THAT CAN BE USED AS DECKING AND GUARD ELEMENTS AS PERMITTED HEREIN, PERMISSIBLE WOOD-PLASTIC COMPOSITES MUST BEAR A LABEL INDICATING ITS PERFORMANCE CRITERIA AND COMPLIANCE WITH ASTM D 7032.

- 3. NAILS SHALL BE RING-SHANKED OR ANNULAR GROOVED.
- 4. SCREWS AND NAILS SHALL BE HOT-DIPPED GALVANIZED, STAINLESS STEEL OR APPROVED FOR USE WITH PRESSURE TREATED LUMBER.
- 5. HARDWARE, E.G., JOIST HANGERS, CAST-IN-PLACE POST ANCHORS, MECHANICAL FASTENERS, SHALL BE GALVANIZED WITH 1.85 OZ/SF OF ZINC (G-185 COATING) OR SHALL BE STAINLESS STEEL. USE PRODUCTS SUCH AS "ZMAX" FROM SIMPSON STRONG-TIE OR "TRIPLE ZINC" AND "GOLD COAT" FROM USP.
- 6. ELECTRICAL RECEPTACLES FOR DECKS SHALL COMPLY WITH THE CURRENTLY APPROVED EDITION OF THE NATIONAL ELECTRICAL CODE.
- 7. LIGHTING FOR DECKS AND EXTERIOR STAIRS SHALL COMPLY WITH IRC 303.7 STAIRWAY ILLUMINATION.
- 8. DECKS CONSTRUCTED IN ACCORDANCE WITH THESE DETAILS ARE NOT APPROVED FOR PRIVACY SCREENS, PLANTERS, BUILT-IN SEATING OR HOT TUB INSTALLATIONS.



POST TO BEAM CONNECTION DETAIL



2'-0"

REBAR ADJUST REBAR

SIZE PER SOIL REQ. (ENGINEER VERIFY)

PICAL DECK AND RAILING DETAIL

ENGINEER STAMP & SEAL



STUDIO TEN DESIGNS



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TEMUVEL @
1776 LITCHFIELD RD SW
SNELLVILLE, GA 20078

PROJECT MANAGER

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SEPTEMBER 23, 2021 SHEET TITLE:

PROPOSED
DECK FRAMING PLANS

ENGINEER DATA

WIND DESIGN:

BASIC WIND SPEED, ULTIMATE

BASIC WIND SPEED, SERVICE

ENCLOSURE CLASSIFICATION

EXPOSURE

C

PISK CATEGORY

RISK CATEGORY II
INTERNAL PRESSURE COEFFICIENT + 0.18

COMPONENTS & CLADDING

AREA ROOF (GROSS, PSF)

5Q.FT ZONE | ZONE 2 ZONE 3 ZONE |, 2 & 3

10.00 - 28.8 -48.4 -72.8 + 16.0

20.00 -28.1 -43.2 -60.3 + 16.0

50.00 -27.1 -36.4 -43.8 + 16.0

100.00 -26.4 -31.3 -31.3 + 16.0

AREA WALLS (GROSS, PSF)

5Q.FT ZONE 4 ZONE 5 ZONE 4&5

10.00 -28.6 -35.2 + 26.4

20.00 -27.4 -32.8 + 25.2

50.00 -25.9 -29.7 + 23.7

100.00 -24.7 -27.4 + 22.5

EDGE DISTANCE, a = 3.2 FT.

SEISMIC DESIGN:

RISK CATEGORY

SITE CLASS

C

IMPORTANCE FACTOR

SPECIAL RESPONSE ACCELERATION

SS = 0.1798

SI = 0.0898

SPECTRAL RESPONSE COEFFICIENTS

SD = 0.1918

SDI = 0.1428

SEISMIC DESIGN CATEGORY

B

RESPONSE MODIFICATION FACTOR (R)

(INTERMEDIATE REINFORCED MASONRY SHEAR WALLS)

OR (R) (INTERMEDIATE REINFORCED MASONRY SHEAR WALLS)
6.5 (LIGHT FRAME WOOD WALLS WITH STRUCTURAL WOOD
SHEAR PANELS)

APACITY 1500 PSF (ASSUME)

LOADS

F 20 PSF (WITH TRIBUTARY REDUCTIONS PER

35 AND LANDINGS 100 PSF

39 AND 1 AND PAN PAN CONTROLLING OF 50 PLF OR 200 LB POINT LO

MATERIALS
POST-INSTALLED ANCHOR RODS
ASTM A 193 GRADE B7 W/ COATING AS SPECIFIED IN ESR-2262 C

CONCRETE (28 DAYS)

FOOTINGS 3000 PSI

WEAR SLAB/ SLAB-ON-GRADE 3000 PSI

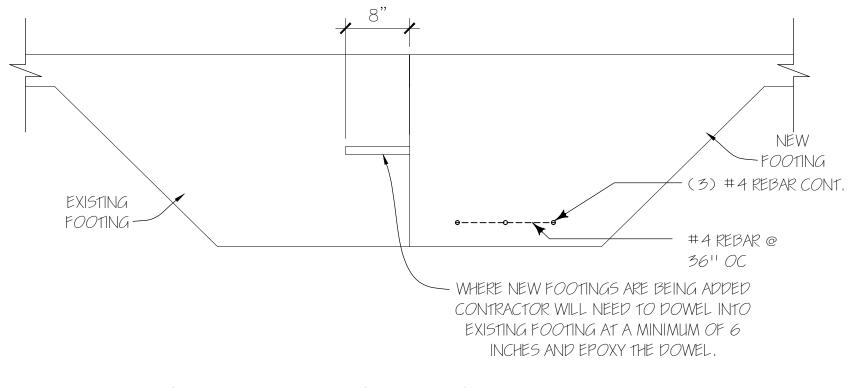
ALL OTHER CONCRETE 3000 PSI

ALL OTHER CONCRETE 3000 PSI
REINFORCING STEEL A615 GRADE
HEADED STUDS A108
WELDED WIRE FABRIC A185

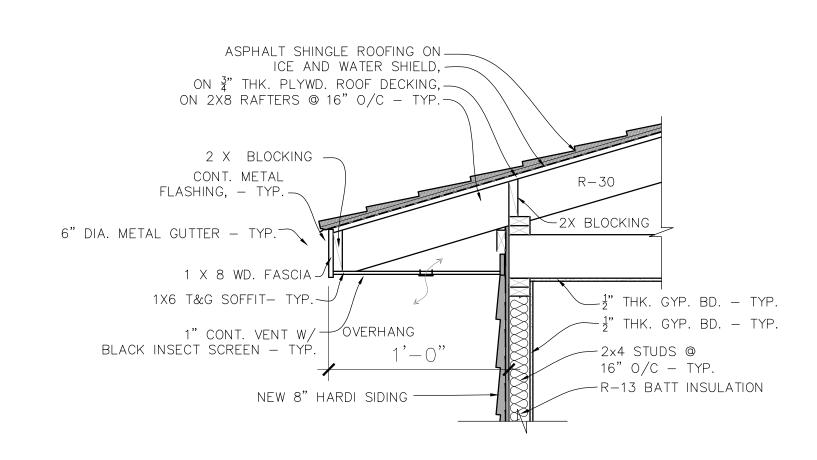
HILTI HIT-RE 500-V3 ADHESIVE ANCHOR SYSTEM (ICC ESR-3814) HILTI HIT-HY 70 ADHESIVE ANCHOE SYSTEM FOE CMU (ICC ESR 2682)

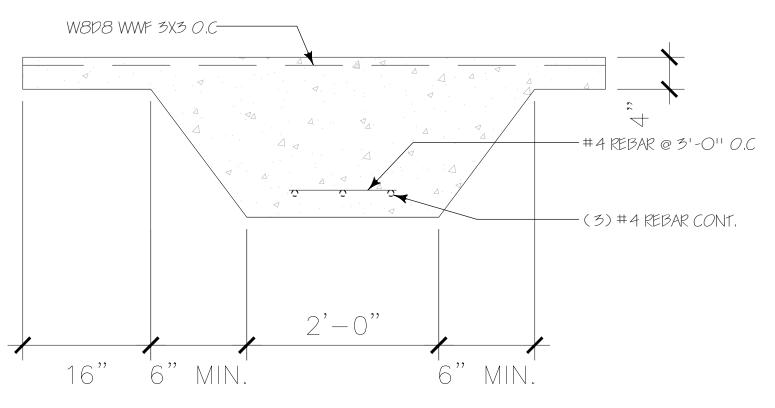
	<u>NO.</u>	SIZE	<u>1YPE</u>	<u>NOTES</u>
	(DI)	3'-0" x 6'-8"	INTERIOR HOLLOW CORE MASONITE	BARN DOOR
	(p2)	2'-4'' x 6'-8''	INTERIOR HOLLOW CORE MASONITE	POCKET DOOR
	03	2'-4'' x 6'-8''	INTERIOR HOLLOW CORE MASONITE	
	D4	2'-0'' x 6'-8''	INTERIOR HOLLOW CORE MASONITE	
7	(P5)	2'-8'' x 6'-8''	INTERIOR HOLLOW CORE MASONITE	POCKET DOOR
	\$6	(2)2'-6" x 6'-8"	EXTERIOR TEMPERED/INSUL, GLASS	
\leq	D7	2'-8" x 6'-8"	INTERIOR HOLLOW CORE MASONITE	
5	(p8)	2'-8'' x 6'-8''	INTERIOR HOLLOW CORE MASONITE	
7	<i>b9</i>	2'-8" x 6'-8"	EXTERIOR TEMPERED/INSUL, GLASS	
	PIO	2'-4" x 6'-8"	INTERIOR HOLLOW CORE MASONITE	
	DII	2'-4" x 6'-8"	INTERIOR HOLLOW CORE MASONITE	

	FIELD VERIFY ALL OPENINGS BEFORE ORDERING WINDOWS OR DOORS						
_ \\\	<u>NO.</u>	<u>SIZE</u>	<u>Q1Y.</u>	<u>1YPE</u>	<u>NOTES</u>		
NO C	(WI)	4'-0'' x 4'-0''	I	FIXED TEMPERED			
N N N N N N N N N N N N N N N N N N N	(W2)	2'-6" x 3'-0"	I	SWING OUT			
	(W3)	3'-0" x 5'-0"	3	DOUBLE HUNG			
	(W4)	2'-6" x 3'-10"	2	DOUBLE HUNG			

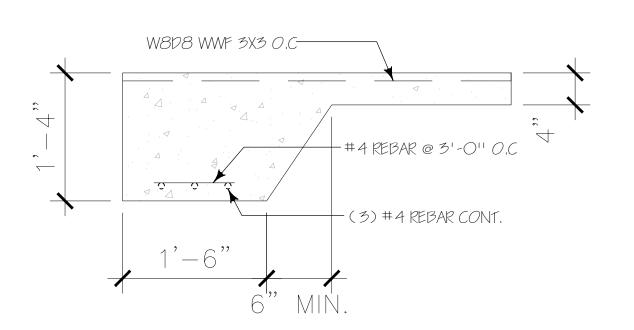














ENGINEER STAMP & SEAL:



STUDIO TEN DESIGNS



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> TTO LITCHFIELD RD SW SNELLVILLE, GA 20078

PROJECT MANAGER

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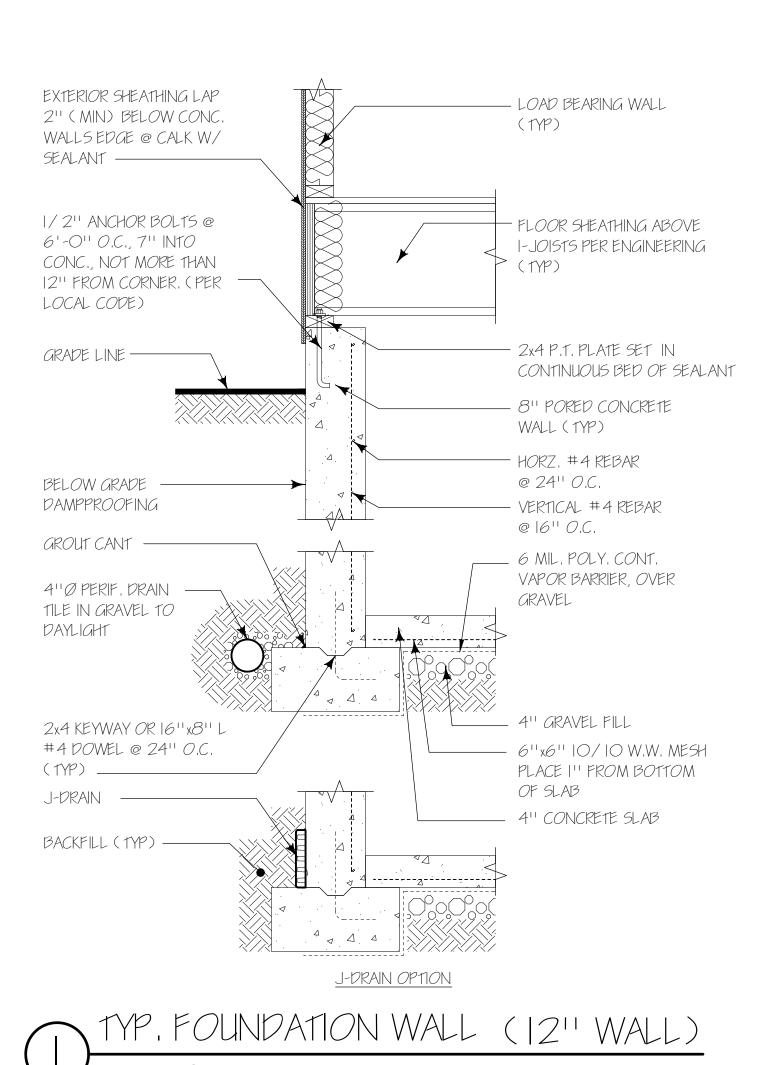
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SEPTEMBER 23, 2021 SHEET TITLE:

PROPOSED

SECTIONS & DETAILS





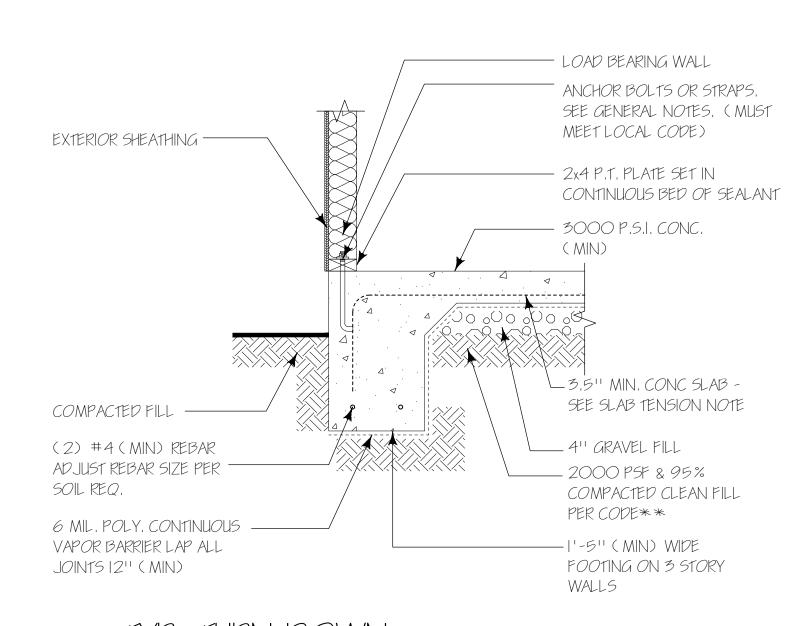


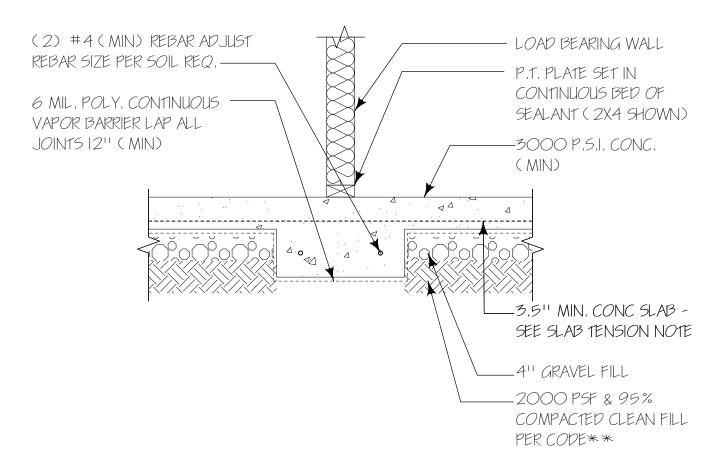


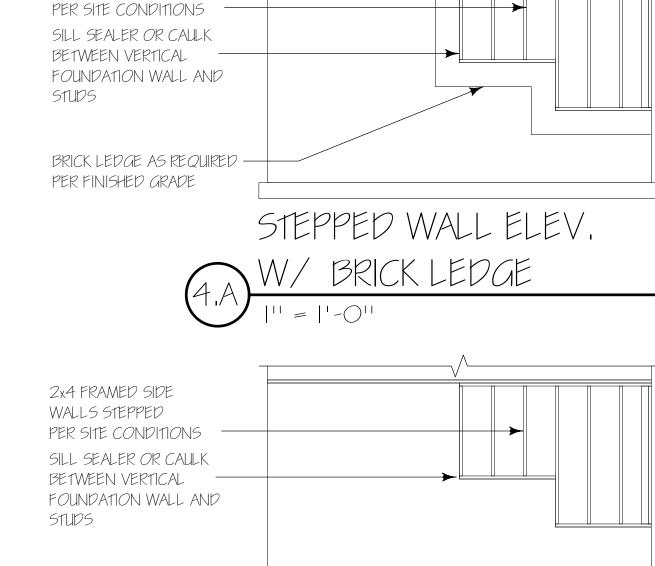
MIN 33" O.C.PER R703.2

** PROVIDE REQUIRED WATER-RESISTIVE

BARRIER OVER SHEATHING PER R703.2





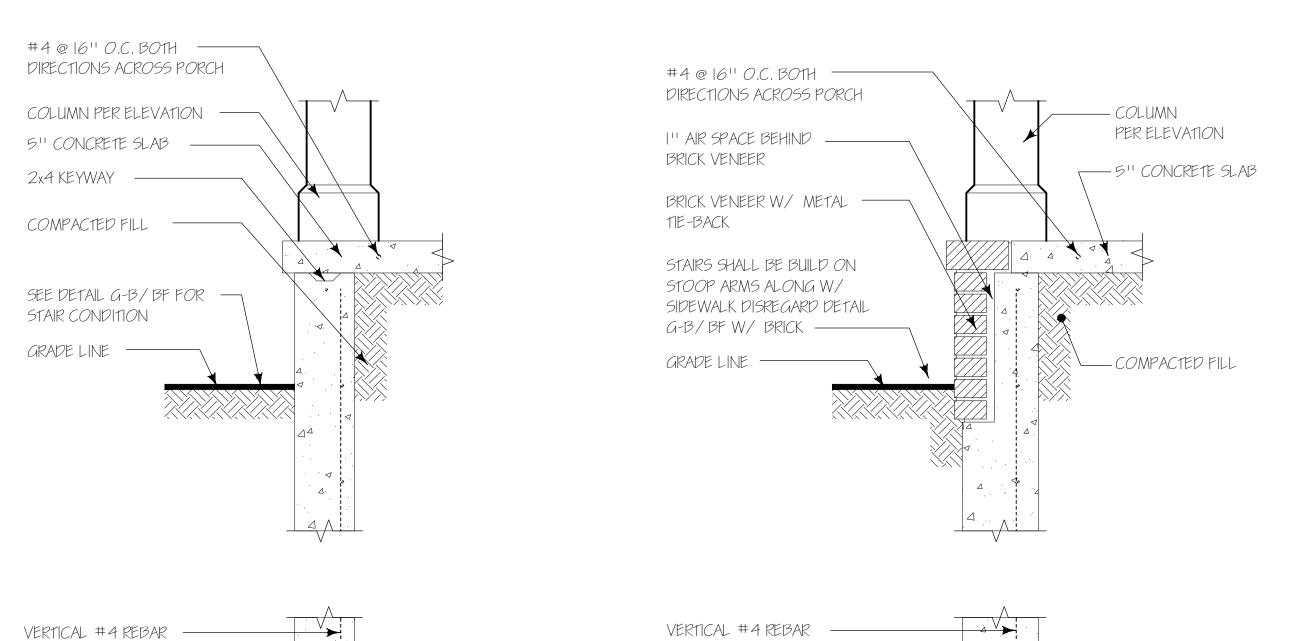


2x4 FRAMED SIDE

WALLS STEPPED

TYP. BR'G WALL (THICKENED SLAB)

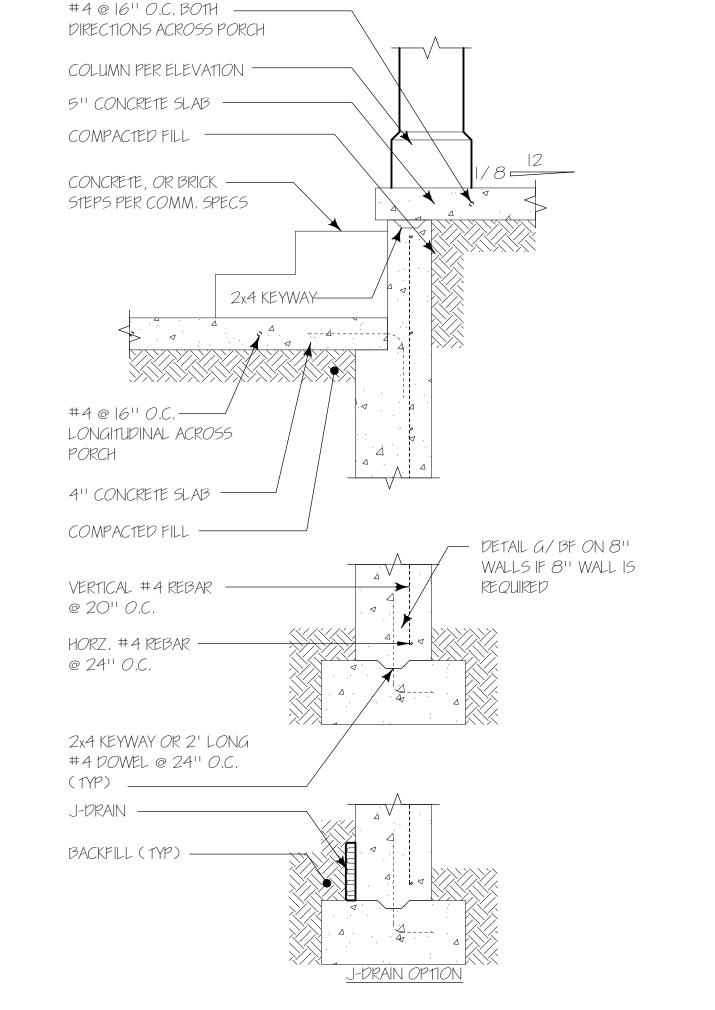


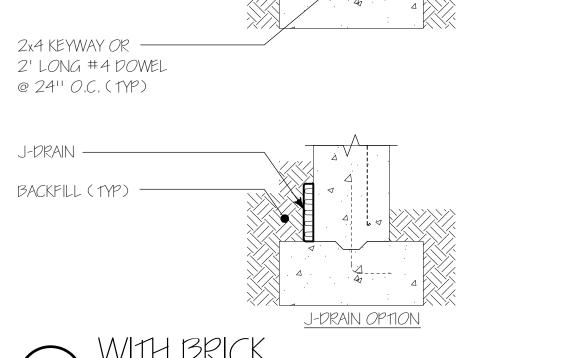


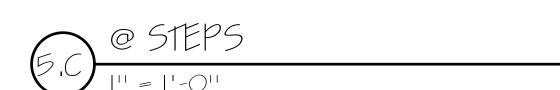
@ 20'' 0.C.

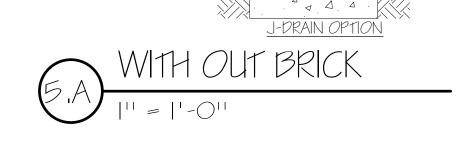
@ 24" O.C.

HORZ. #4 REBAR -









@ 1611 O.C.

@ 24" O.C.

(1YP)

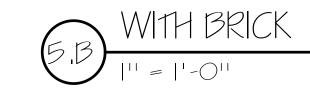
J-DRAIN

BACKFILL (TYP)

HORZ. #4 REBAR

2x4 KEYWAY *O*R 16''x8'' L

#4 DOWEL @ 24" O.C.







ENGINEER STAMP & SEAL





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PROJECT MANAGER

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DOCUMENT PHASE:



SEPTEMBER 23, 2021 SHEET TITLE:

PROPOSED SECTIONS & DETAILS

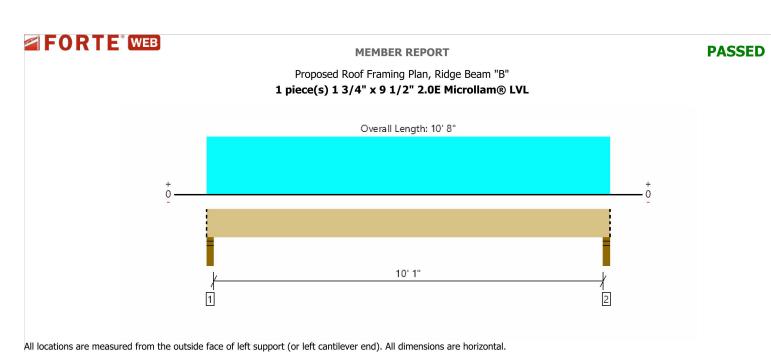
FORTE WEB JOB SUMMARY REPORT

I L W	1776 Litchfield Road SW	
Plan		
Results	Current Solution	Comments
Passed	2 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL	
Passed	2 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL	
Passed	1 piece(s) 1 3/4" x 9 1/2" 2.0E Microllam® LVL	
Passed	1 piece(s) 1 3/4" x 9 1/2" 2.0E Microllam® LVL	
Passed	1 piece(s) 1 3/4" x 9 1/2" 2.0E Microllam® LVL	
Passed	1 piece(s) 2 x 6 SPF No.1/No.2 @ 16" OC	
Framing Plan		
Results	Current Solution	Comments
Passed	2 piece(s) 2 x 8 SPF No.1/No.2	
Passed	2 piece(s) 2 x 8 SPF No.1/No.2	
Passed	1 piece(s) 2 x 6 SPF No.1/No.2 @ 16" OC	
raming Plan		
Results	Current Solution	Comments
Passed	4 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL	
Passed	1 piece(s) W12X26 (A992) ASTM Steel	
Passed	2 piece(s) 2 x 8 SPF No.1/No.2	
Passed	1 piece(s) 11 7/8" TJI® 230 @ 16" OC	
ing Plan		
Results	Current Solution	Comments
Passed	1 piece(s) 2 x 12 SPF No.1/No.2 @ 16" OC	
Plan		
Results	Current Solution	Comments
Passed	2 piece(s) 2 x 10 SPF No.1/No.2	
Passed	2 piece(s) 2 x 10 SPF No.1/No.2	
Passed	1 piece(s) 2 x 8 SPF No.1/No.2 @ 16" OC	
	Plan Results Passed Passed	Plan Results Current Solution Passed 2 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL Passed 2 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL Passed 1 piece(s) 1 3/4" x 9 1/2" 2.0E Microllam® LVL Passed 1 piece(s) 1 3/4" x 9 1/2" 2.0E Microllam® LVL Passed 1 piece(s) 1 3/4" x 9 1/2" 2.0E Microllam® LVL Passed 1 piece(s) 1 3/4" x 9 1/2" 2.0E Microllam® LVL Passed 1 piece(s) 2 x 6 SPF No.1/No.2 @ 16" OC Passed 1 piece(s) 2 x 6 SPF No.1/No.2 @ 16" OC Passed 2 piece(s) 2 x 8 SPF No.1/No.2 Passed 2 piece(s) 2 x 8 SPF No.1/No.2 Passed 1 piece(s) 2 x 6 SPF No.1/No.2 @ 16" OC Passed 1 piece(s) 2 x 6 SPF No.1/No.2 @ 16" OC Passed 1 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL Passed 4 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL Passed 1 piece(s) 2 x 8 SPF No.1/No.2 Passed 1 piece(s) 1 1 7/8" TJI® 2.0E Microllam® LVL Passed 1 piece(s) 1 1 7/8" TJI® 2.0E Microllam® LVL Passed 1 piece(s) 1 1 7/8" TJI® 2.0E Microllam® LVL Passed 1 piece(s) 2 x 8 SPF No.1/No.2 Passed 1 piece(s) 2 x 8 SPF No.1/No.2 Passed 1 piece(s) 2 x 8 SPF No.1/No.2 Passed 1 piece(s) 2 x 8 SPF No.1/No.2 @ 16" OC Passed 1 piece(s) 2 x 12 SPF No.1/No.2 @ 16" OC Passed 1 piece(s) 2 x 12 SPF No.1/No.2 @ 16" OC Passed 2 piece(s) 2 x 10 SPF No.1/No.2 2 piece(s) 2 x 10 SPF No.

rteWEB Software Operator	Job Notes	
William Hamilton spection Wizards LLC 78) 770-4079 amilton@InspectionWizards.com		Weyerhaeuse
78) 770-4079		Weyerh

11/19/2021 9:49:03 AM UTC ForteWEB v3.2 File Name: 1776 Litchfield Road SW

Page 1 / 18



			,			
Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)	System : Roof
Member Reaction (lbs)	1226 @ 2"	2603 (3.50")	Passed (47%)		1.0 D + 1.0 Lr (All Spans)	Member Type : Flush Bear Building Use : Residential
Shear (lbs)	977 @ 1' 1"	3948	Passed (25%)	1.25	1.0 D + 1.0 Lr (All Spans)	Building Code : IBC 2018
Moment (Ft-lbs)	3068 @ 5' 4"	7359	Passed (42%)	1.25	1.0 D + 1.0 Lr (All Spans)	Design Methodology : ASI
Live Load Defl. (in)	0.151 @ 5' 4"	0.344	Passed (L/821)		1.0 D + 1.0 Lr (All Spans)	Member Pitch: 0/12
Total Load Defl. (in)	0.257 @ 5' 4"	0.517	Passed (L/482)		1.0 D + 1.0 Lr (All Spans)	
						=

 Deflection criteria: LL (L/360) and TL (L/240). • Allowed moment does not reflect the adjustment for the beam stability factor.

	Bearing Length Loads to Supports (lbs)						
Supports	Total	Available	Required	Dead	Roof Live	Total	Accessories
1 - Stud wall - SPF	3.50"	3.50"	1.65"	506	720	1226	Blocking
2 - Stud wall - SPF	3.50"	3.50"	1.65"	506	720	1226	Blocking

 Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed. 						
Lateral Bracing	Bracing Intervals	Comments				
Top Edge (Lu)	10' 8" o/c					
Bottom Edge (Lu)	10' 8" o/c					
•Maximum allowable bracing intervals based on applied load.						

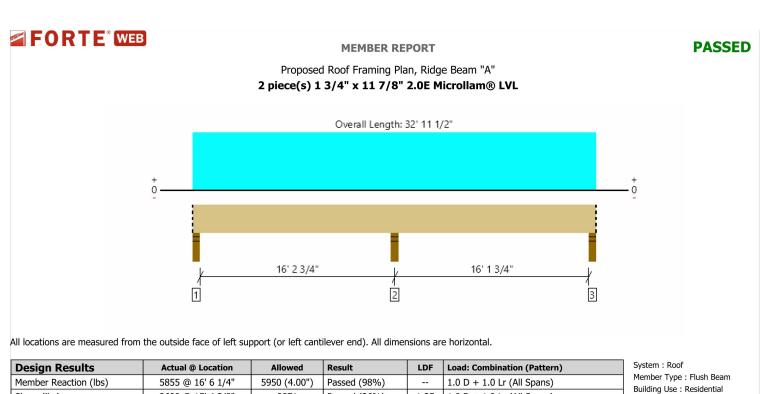
			Dead	Roof Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(non-snow: 1.25)	Comments
0 - Self Weight (PLF)	0 to 10' 8"	N/A	4.8		
1 - Uniform (PSF)	0 to 10' 8" (Front)	9'	10.0	15.0	Default Load

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes	
C William Hamilton Inspection Wizards LLC (678) 770-4079 Bhamilton@InspectionWizards.com		Weyerhae

11/19/2021 9:49:03 AM UTC ForteWEB v3.2, Engine: V8.2.0.17, Data: V8.1.0.16 File Name: 1776 Litchfield Road SW Page 4 / 18



			,			
Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)	System : Roof
Member Reaction (lbs)	5855 @ 16' 6 1/4"	5950 (4.00")	Passed (98%)		1.0 D + 1.0 Lr (All Spans)	Member Type : Flush BeaBuilding Use : Residentia
Shear (lbs)	2600 @ 15' 4 3/8"	9871	Passed (26%)	1.25	1.0 D + 1.0 Lr (All Spans)	Building Code : IBC 2018
Moment (Ft-lbs)	-9551 @ 16' 6 1/4"	22310	Passed (43%)	1.25	1.0 D + 1.0 Lr (All Spans)	Design Methodology : AS
Live Load Defl. (in)	0.167 @ 7' 6 5/8"	0.818	Passed (L/999+)		1.0 D + 1.0 Lr (Alt Spans)	Member Pitch : 0/12
Total Load Defl. (in)	0.262 @ 7' 4 3/8"	1.090	Passed (L/749)		1.0 D + 1.0 Lr (Alt Spans)	
Deflection criteria: LL (L/240) and	I TL (L/180).		•			
· Allowed moment does not reflect	the adjustment for the beam s	tability factor.				

	Bearing Length		Loads to Supports (lbs)				
Supports	Total	Available	Required	Dead	Roof Live	Total	Accessories
1 - Stud wall - SPF	3.50"	3.50"	1.50"	771	1124	1895	Blocking
2 - Stud wall - SPF	4.00"	4.00"	3.94"	2490	3364	5854	None
3 - Stud wall - SPF	3.50"	3.50"	1.50"	764	1118	1882	Blocking
Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.							

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	25' 8" o/c	
Bottom Edge (Lu)	15' 8" o/c	
•Maximum allowable bracing inter-	vals based on applied load.	

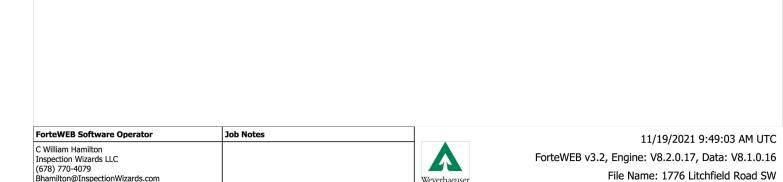
			Dead	Roof Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(non-snow: 1.25)	Comments
0 - Self Weight (PLF)	0 to 32' 11 1/2"	N/A	12.1		
1 - Uniform (PSF)	0 to 32' 11 1/2" (Front)	11'	10.0	15.0	Default Load

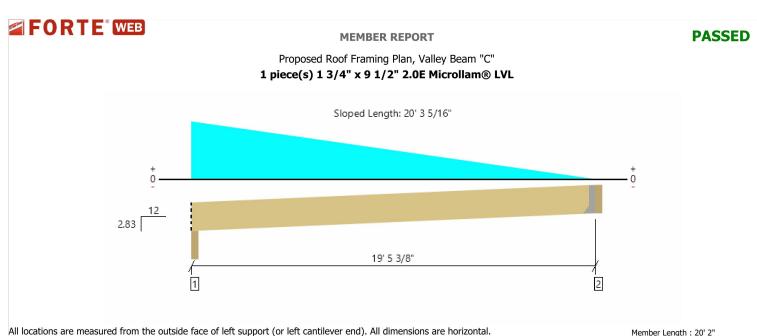
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Maximum allowable bracing intervals based on applied load.





all locations are measured from the outside race of felt support (of felt cartalever end). All difficulties are notizontal.							
Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)	System : Roof	
Member Reaction (lbs)	1291 @ 2"	2603 (3.50")	Passed (50%)		1.0 D + 1.0 Lr (All Spans)	Member Type : Flush Beam Building Use : Residential	
Shear (lbs)	1089 @ 1' 3/4"	3948	Passed (28%)	1.25	1.0 D + 1.0 Lr (All Spans)	Building Code : IBC 2018	
Moment (Ft-lbs)	4717 @ 8' 4 9/16"	7359	Passed (64%)	1.25	1.0 D + 1.0 Lr (All Spans)	Design Methodology : ASD	
Live Load Defl. (in)	0.584 @ 9' 5 5/16"	0.990	Passed (L/407)		1.0 D + 1.0 Lr (All Spans)	Member Pitch : 2.83/12	
Total Load Defl. (in)	1.335 @ 9' 5 9/16"	1.321	Passed (L/178)		1.0 D + 1.0 Lr (All Spans)		
Deflection criteria: LL (L/240) and	d TL (L/180).						
· Allowed moment does not reflect	the adjustment for the beam s	tability factor.					

	Bearing Length			Loads t	o Supports (
Supports	Total	Available	Required	Dead	Roof Live	Total	Accessories	
1 - Beveled Plate - SPF	3.50"	3.50"	1.74"	719	572	1291	Blocking	
2 - Hanger on 9 1/2" SPF beam	3.50"	Hanger ¹	1.50"	375	279	654	See note ¹	
Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.								
At hanger supports, the Total Bearing dimens	ion is equal to	the width of	the material t	hat is suppor	ting the hange	er		

• ¹ See Connector grid below for	additional information and/or requirem	ents.
Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	7' 2" o/c	
Bottom Edge (Lu)	20' o/c	

Connector: Simpson Strong-	Гіе					
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
2 - Face Mount Hanger	LSSR1.81Z	1.88"	N/A	14-10dx2.5	12-10dx1.5	
Refer to manufacturer notes and instructi	ons for proper installation and use	of all connectors.				•

		Dead		Roof Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(non-snow: 1.25)	Comments
0 - Self Weight (PLF)	0 to 19' 5 3/8"	N/A	4.8		
1 - Tapered (PLF)	0 to 19' 5 3/8"	N/A	99.8 to 0.0		Generated from Roof Geometry

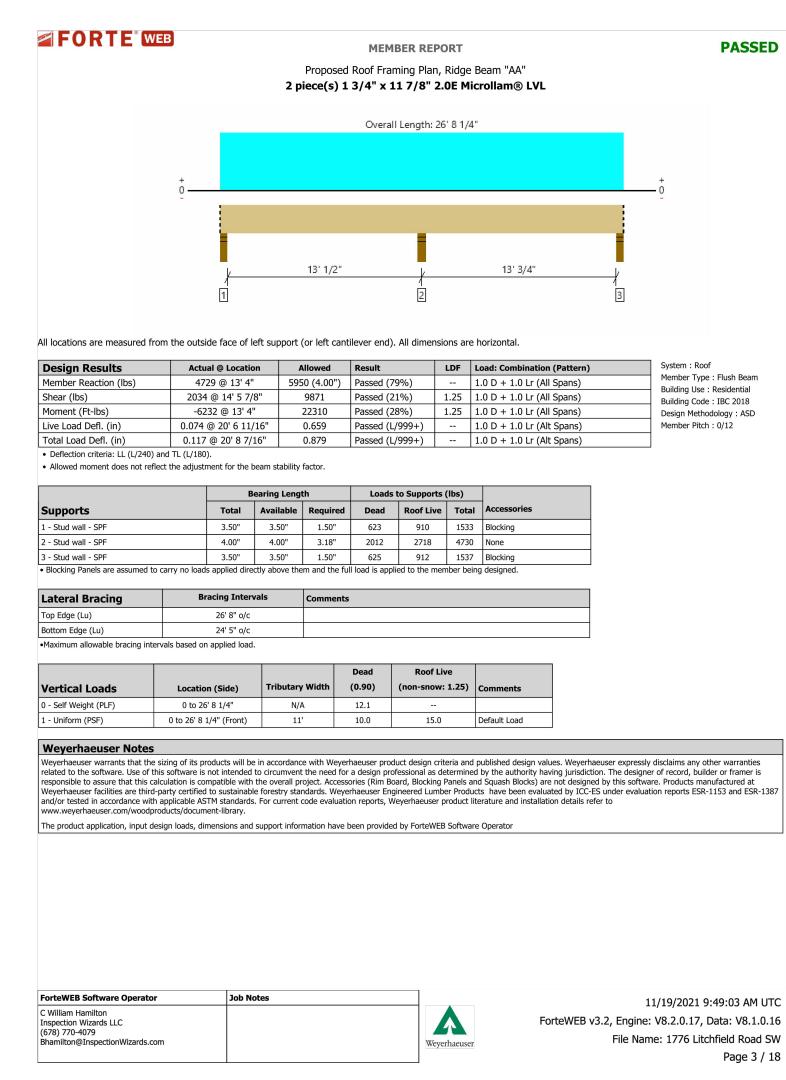
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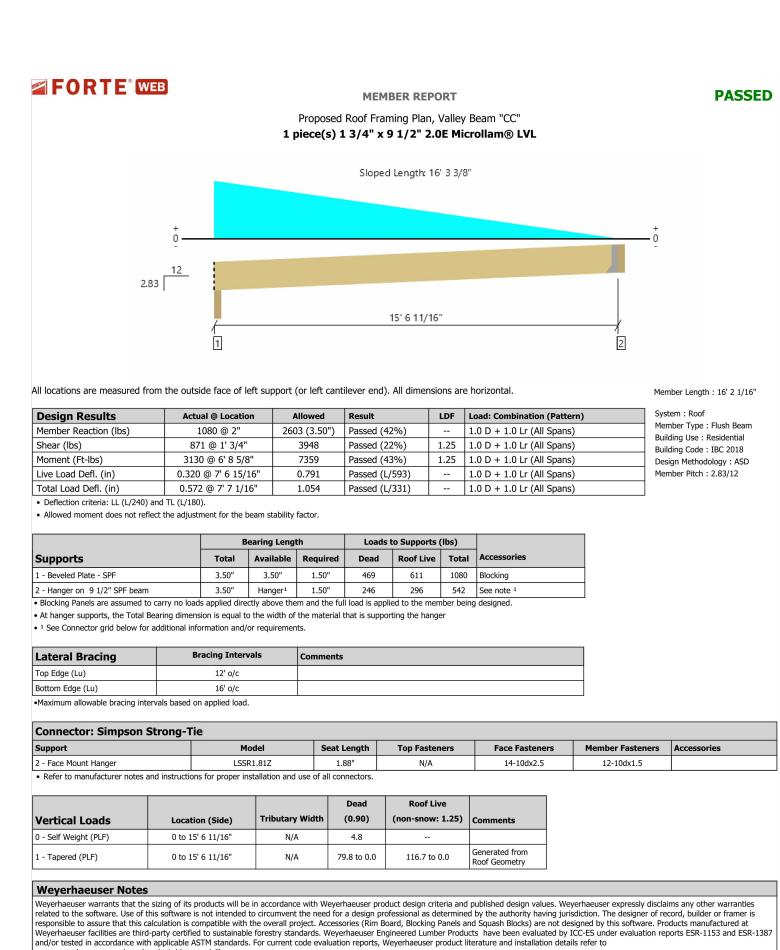
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Page 2 / 18





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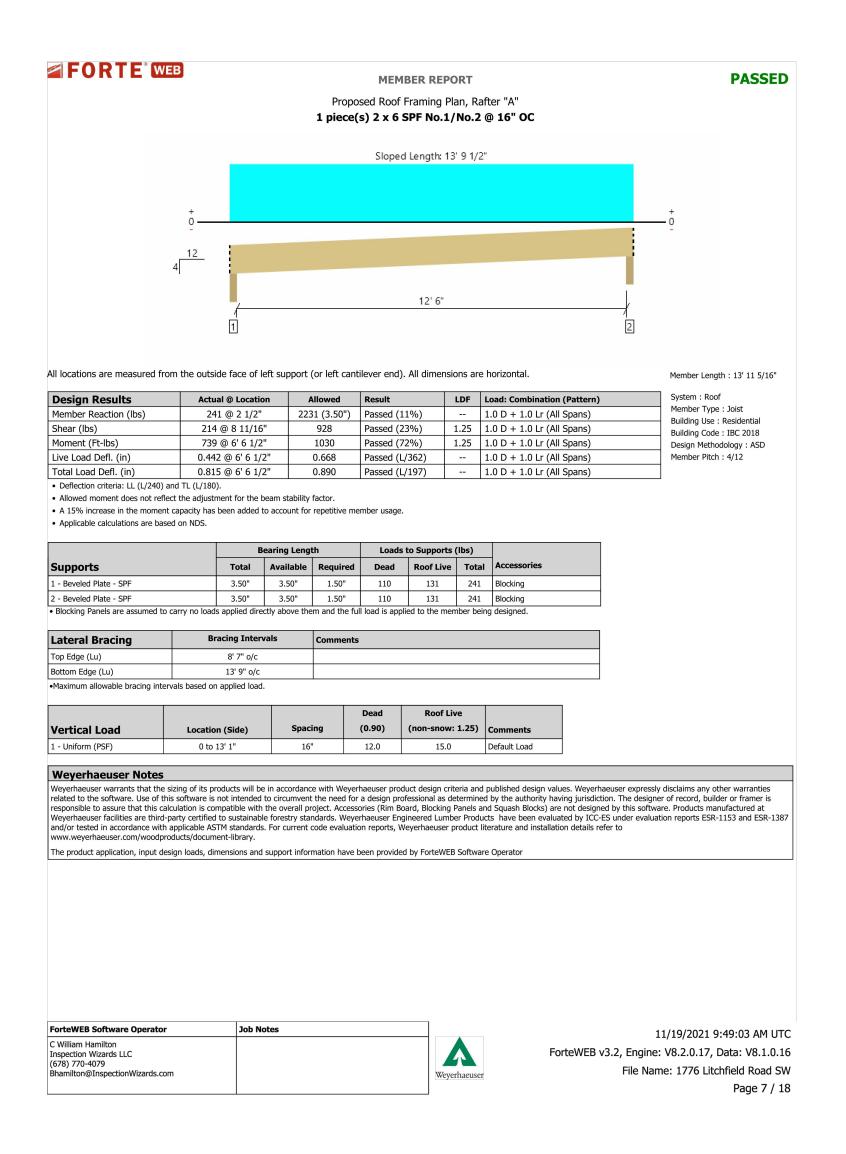


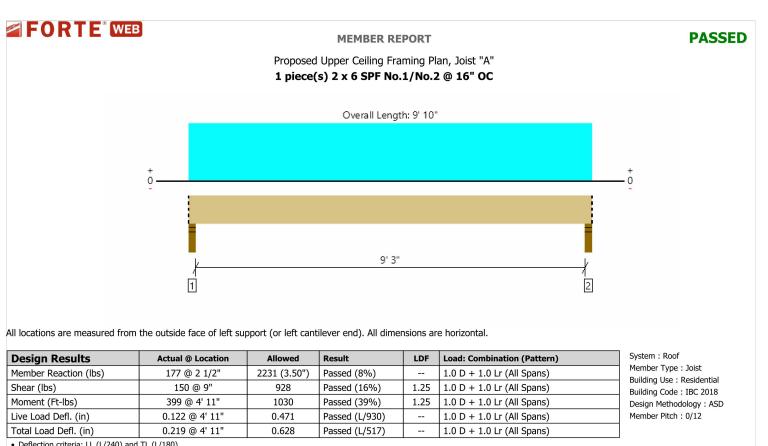
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Page 6 / 18

File Name: 1776 Litchfield Road SW

ForteWEB v3.2, Engine: V8.2.0.17, Data: V8.1.0.16





• Deflection criteria: LL (L/240) and TL (L/180). · Allowed moment does not reflect the adjustment for the beam stability factor.

• A 15% increase in the moment capacity has been added to account for repetitive member usage. • Applicable calculations are based on NDS.

	B	Bearing Length Loads to Supports (lbs)					
Supports	Total	Available	Required	Dead	Roof Live	Total	Accessories
1 - Stud wall - SPF	3.50"	3.50"	1.50"	79	98	177	Blocking
2 - Stud wall - SPF	3.50"	3.50"	1.50"	79	98	177	Blocking

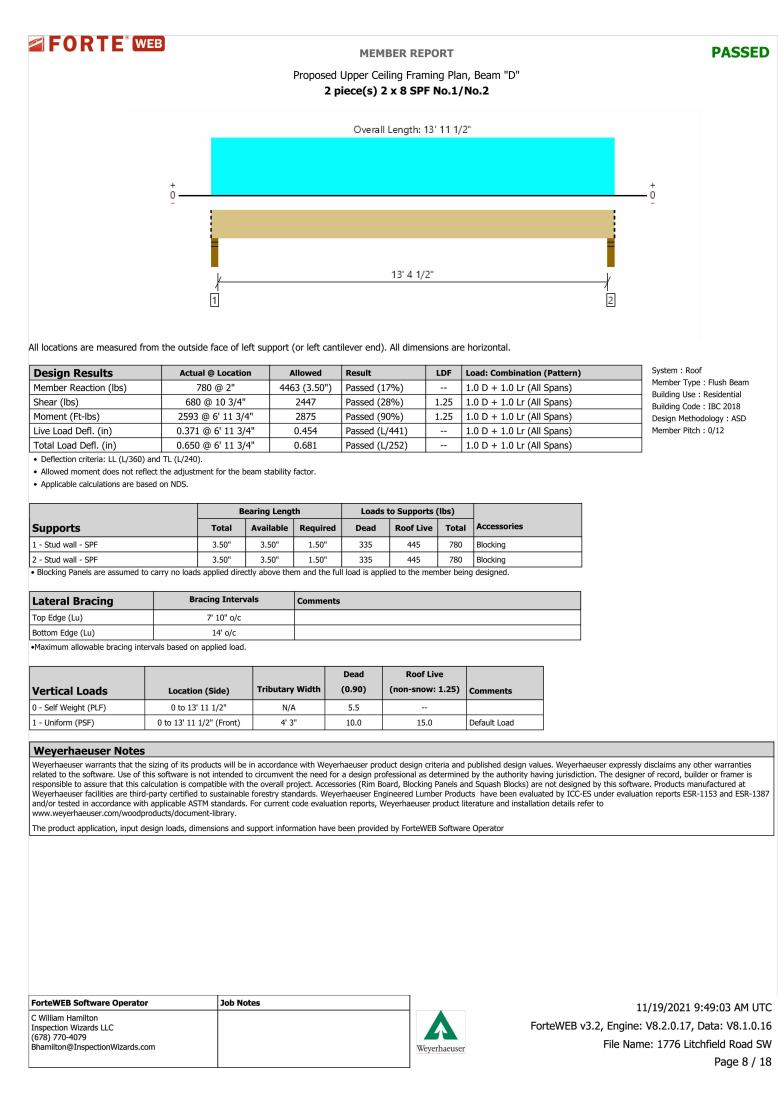
Lateral Bracing	Bracing Intervals	Comme	ents		
Top Edge (Lu)	9' 10" o/c				
Bottom Edge (Lu)	9' 10" o/c				
•Maximum allowable bracing interv	vals based on applied load.				

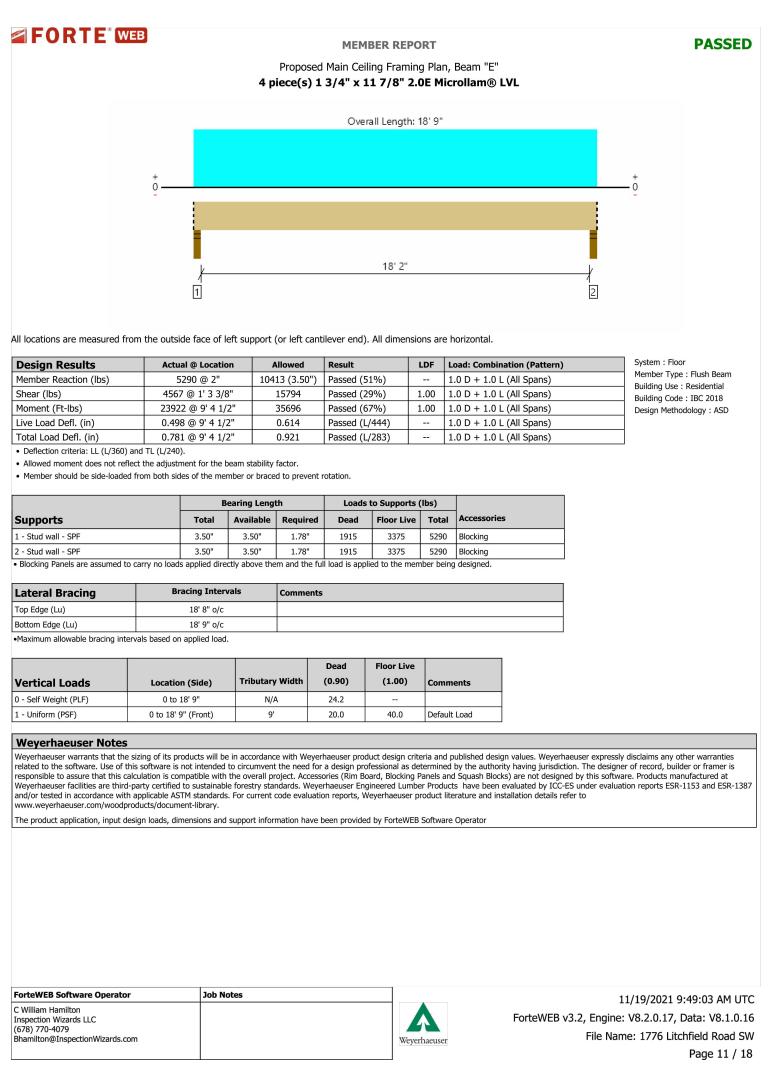
Dead Roof Live Spacing (0.90) (non-snow: 1.25) Comments Vertical Load Location (Side) 0 to 9' 10" 16" 12.0 15.0 Default Load

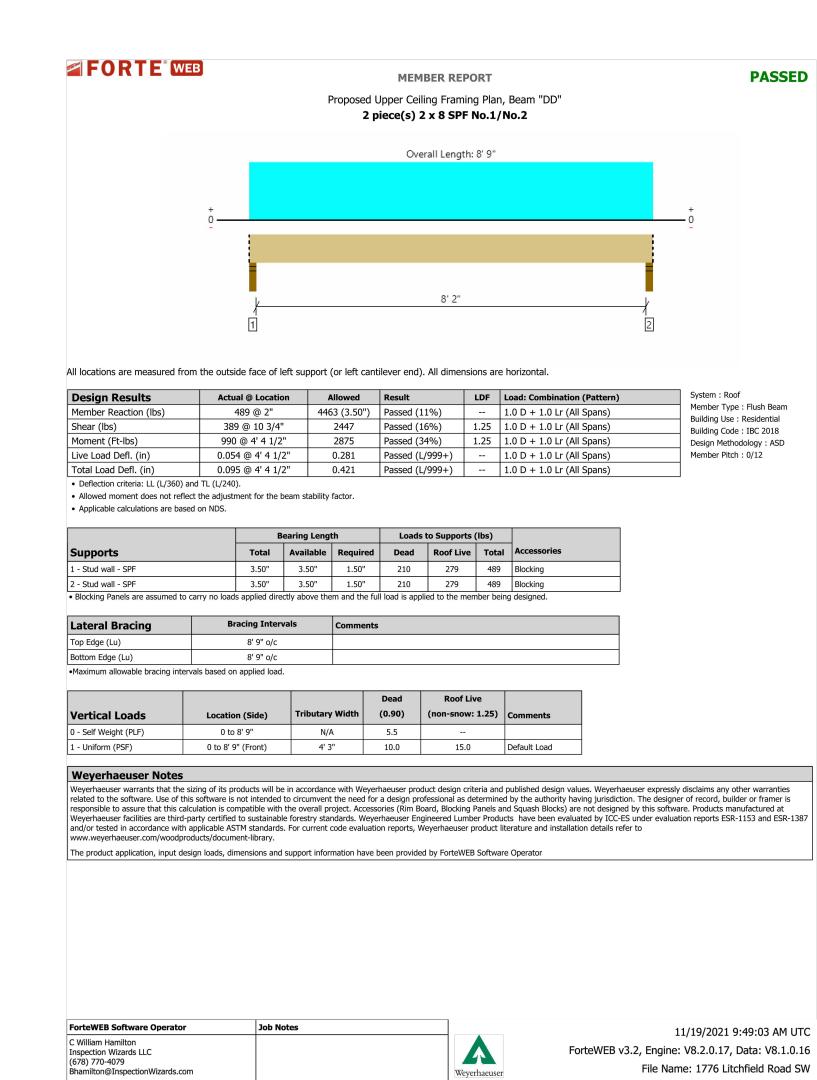
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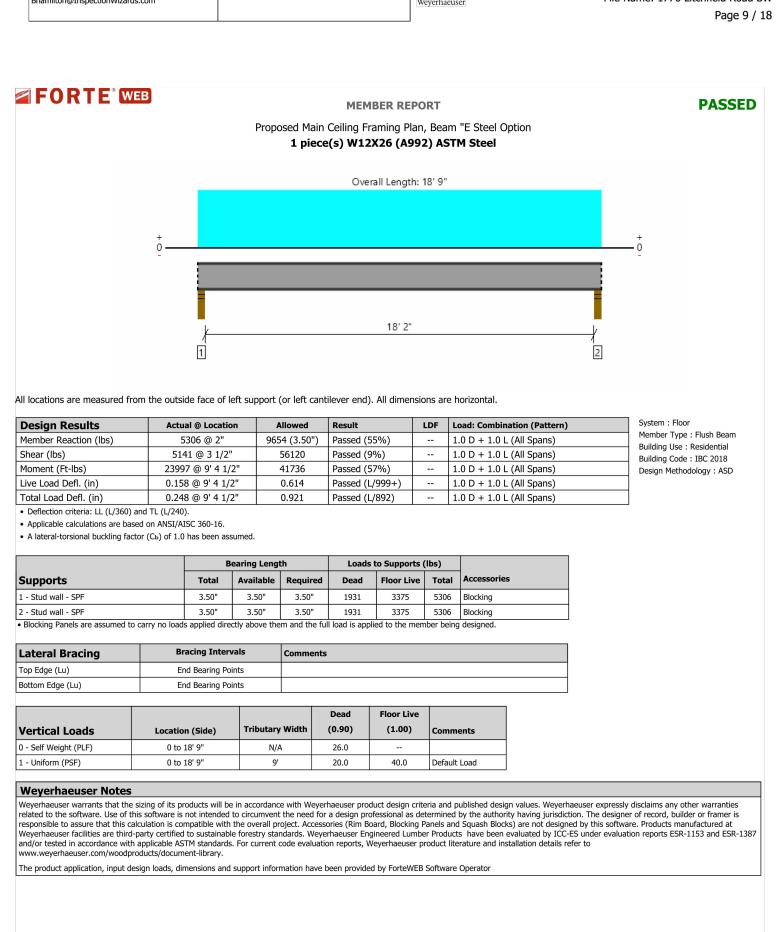
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rteWEB Software Operator Villiam Hamilton pection Wizards LLC '8) 770-4079 amilton@InspectionWizards.com	Job Notes	Weyerhaeuser	11/19/2021 9:49:03 AM UTC ForteWEB v3.2, Engine: V8.2.0.17, Data: V8.1.0.16 File Name: 1776 Litchfield Road SW Page 10 / 18









11/19/2021 9:49:03 AM UTC

Page 12 / 18

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Job Notes





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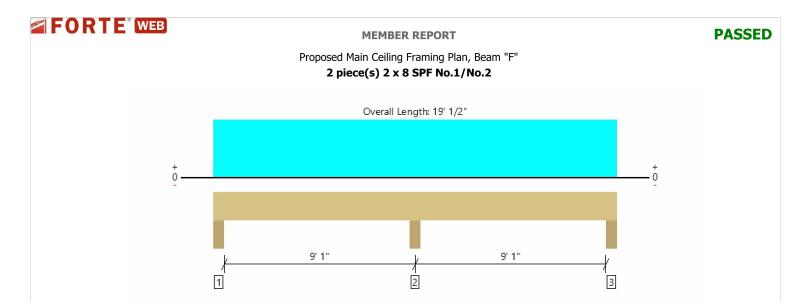
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All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)	System : Roof
Member Reaction (lbs)	2653 @ 9' 6 1/4"	6694 (5.25")	Passed (40%)		1.0 D + 1.0 Lr (All Spans)	Member Type : Flush Beam Building Use : Residential
Shear (lbs)	1137 @ 10' 4 1/8"	2447	Passed (46%)	1.25	1.0 D + 1.0 Lr (All Spans)	Building Code : IBC 2018
Moment (Ft-lbs)	-2443 @ 9' 6 1/4"	2875	Passed (85%)	1.25	1.0 D + 1.0 Lr (All Spans)	Design Methodology : ASD
Live Load Defl. (in)	0.091 @ 14' 6 13/16"	0.307	Passed (L/999+)		1.0 D + 1.0 Lr (Alt Spans)	Member Pitch: 0/12
Total Load Defl. (in)	0.139 @ 14' 8"	0.460	Passed (L/793)		1.0 D + 1.0 Lr (Alt Spans)	
D (1 1) 11 11 (1 (0.60) 1	TI (1 (0 10)					

• Deflection criteria: LL (L/360) and TL (L/240). • Allowed moment does not reflect the adjustment for the beam stability factor.

Applicable calculations are based on NDS.

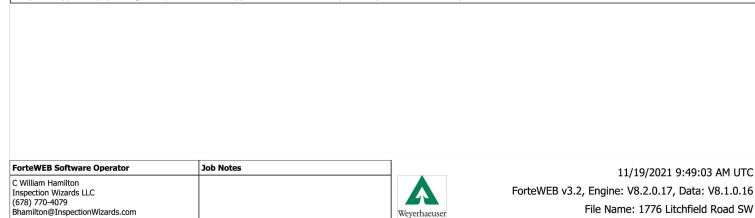
		Bearing Length			Loads to Supports (lbs)				
Supports		Total	Available	Required	Dead	Roof Live	Total	Accessories	
1 - Column - SPF		5.25"	5.25"	1.50"	360	547	907	None	
2 - Column - SPF		5.25"	5.25"	2.08"	1099	1554	2653	None	
3 - Column - SPF		5.25"	5.25"	1.50"	360	547	907	None	
Lateral Bracing	Bı	racing Inter	vals	Comments					

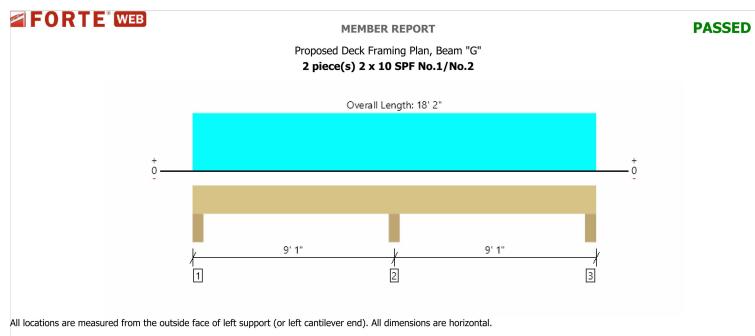
Bottom Edge (Lu) 9' 6" o/c Maximum allowable bracing intervals based on applied load

			Dead	Roof Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(non-snow: 1.25)	Comments
0 - Self Weight (PLF)	0 to 19' 1/2"	N/A	5.5		
1 - Uniform (PSF)	0 to 19' 1/2" (Front)	9'	10.0	15.0	Default Load

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System ; Floor

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)	System . Hoor
Member Reaction (lbs)	3366 @ 9' 1"	6694 (5.25")	Passed (50%)		1.0 D + 1.0 L (All Spans)	Member Type : Flush Beam Building Use : Residential
Shear (lbs)	1379 @ 10' 7/8"	2498	Passed (55%)	1.00	1.0 D + 1.0 L (All Spans)	Building Code : IBC 2018
Moment (Ft-lbs)	-2952 @ 9' 1"	3431	Passed (86%)	1.00	1.0 D + 1.0 L (All Spans)	Design Methodology : ASD
Live Load Defl. (in)	0.068 @ 4' 5 3/4"	0.219	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)	
Total Load Defl. (in)	0.089 @ 4' 4 3/8"	0.439	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)	

 Deflection criteria: LL (L/480) and TL (L/240). Allowed moment does not reflect the adjustment for the beam stability factor.

Applicable calculations are based on NDS.

		Bearing Length			Loads to Supports (lbs)			
Supports		Total	Available	Required	Dead	Floor Live	Total	Accessories
1 - Column - SPF		5.25"	5.25"	1.50"	385	830/-110	1215/- 110	None
2 - Column - SPF		5.25"	5.25"	2.64"	1173	2193	3366	None
3 - Column - SPF		5.25"	5.25"	1.50"	385	830/-110	1215/- 110	None
		•						•
Lateral Bracing	Bracing Intervals		Comments					
Top Edge (Lu)	17' 5" o/c							
Bottom Edge (Lu)	9' 9" o/c							
•Maximum allowable bracing inte	rvals based or	applied load		•				

			Dead	Floor Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	0 to 18' 2"	N/A	7.0		
1 - Uniform (PSF)	0 to 18' 2" (Front)	5'	20.0	40.0	Default Load

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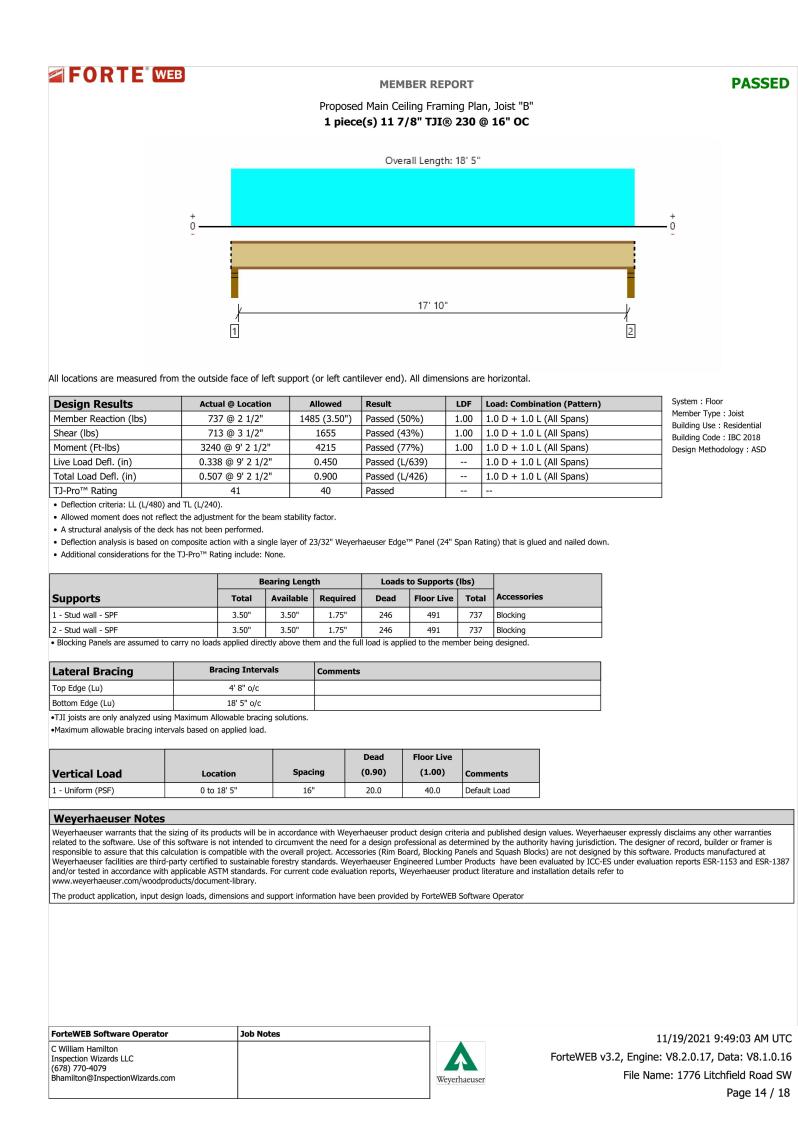
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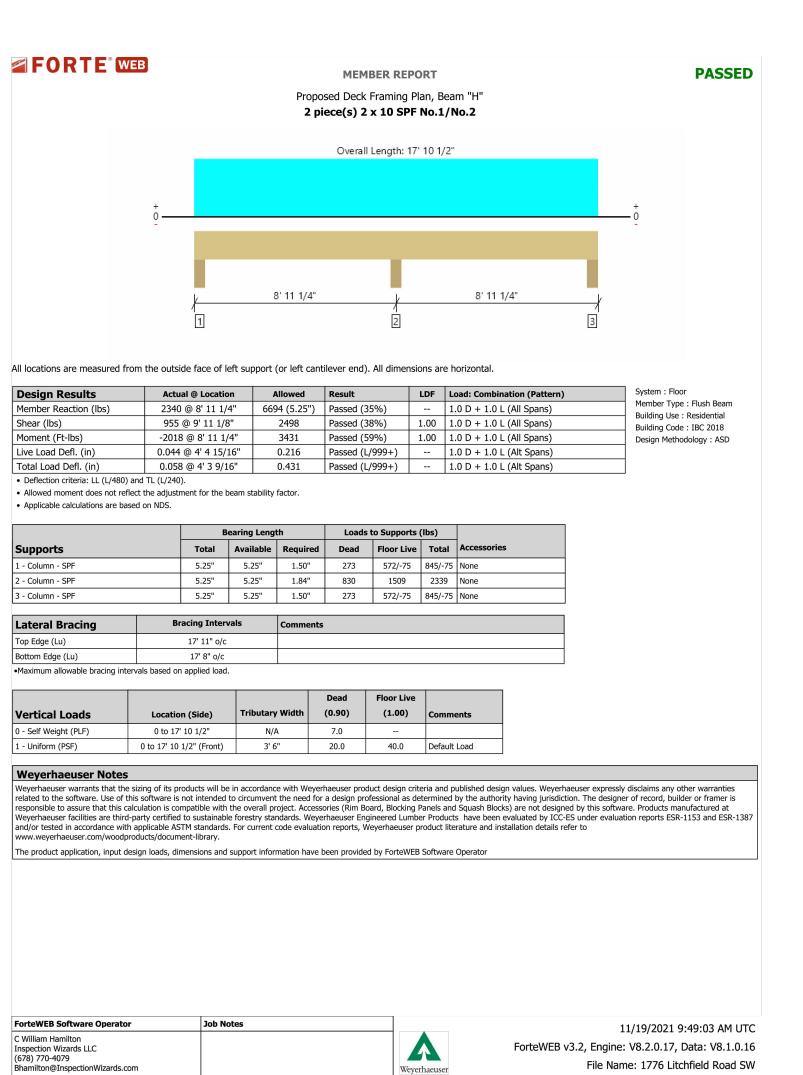
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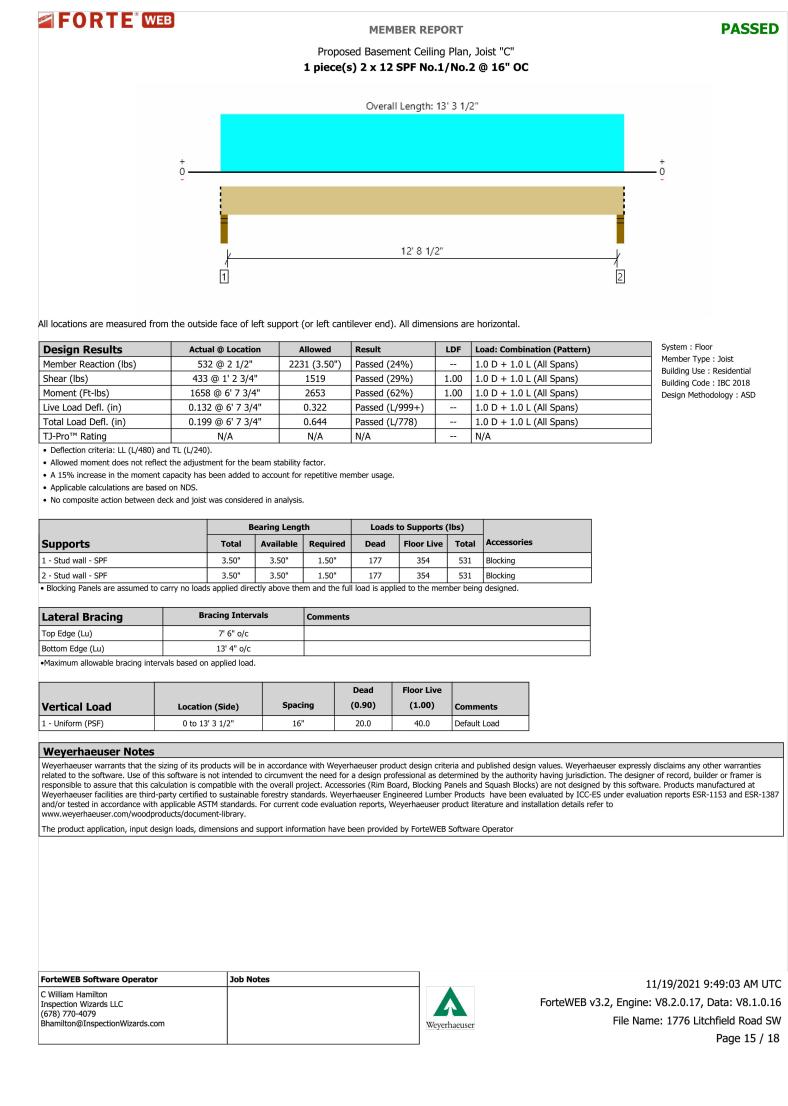
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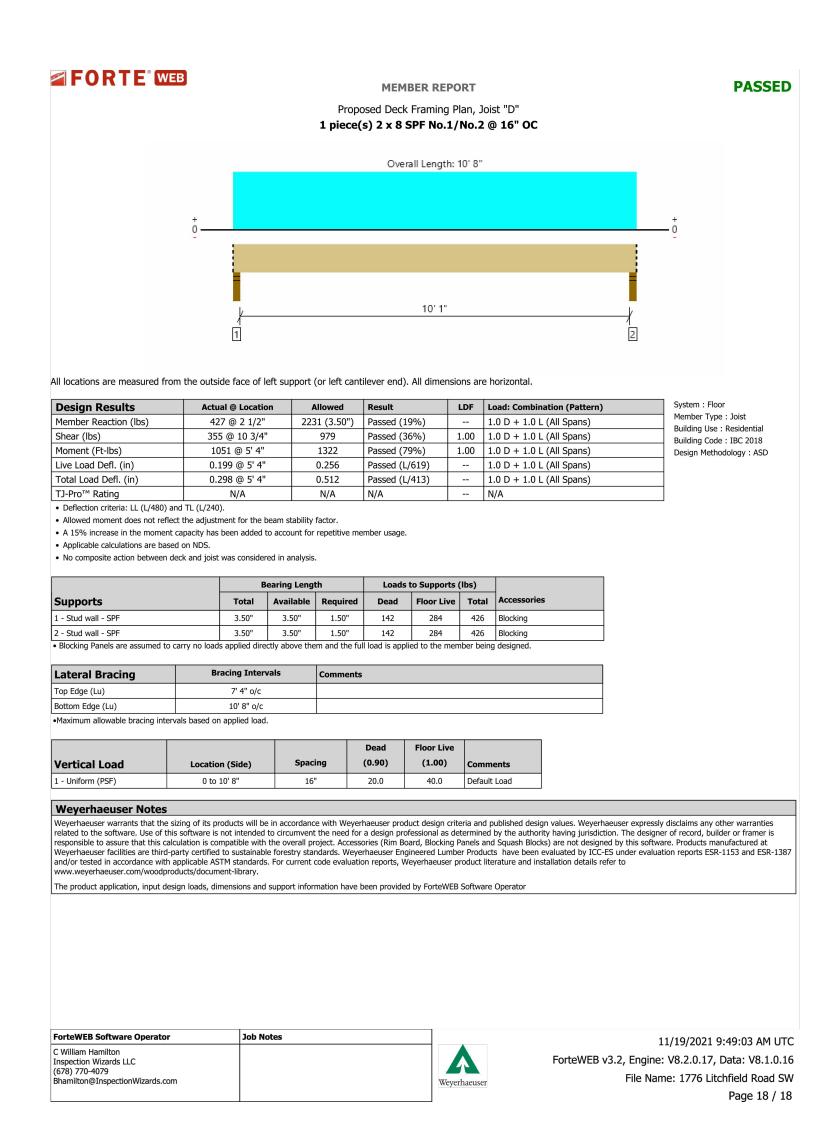
Page 13 / 18

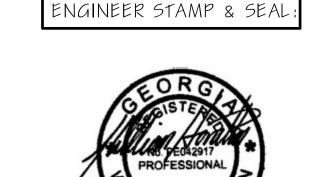




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